



Recyclage optimal des agrégats de béton bitumineux dans les chaussées à faible trafic
Optimales Recycling von Ausbauasphalt auf verkehrsschwachen Straßen

01/11/2016 - 31/12/2020

FINAL ANNUAL MEETING

02/12/2020

Summary

- Materials characterisation (RAP & aggregates)
A. FEESER (Cerema)
- Materials characterisation (bitumen) & Behaviour (MPO & CBR tests)
H. HERB (HsKA)
- Mechanical behaviour (GSC & ITS tests)
A. FEESER (Cerema)
- Mechanical behaviour (Repeated Load Triaxial tests)
L. GAILLARD (INSA)
- Medium Scale Testing
C. RAAB (Empa)

PART I

Materials characterisation (Aggregates & RAP)

Arnaud Feeser, Cerema

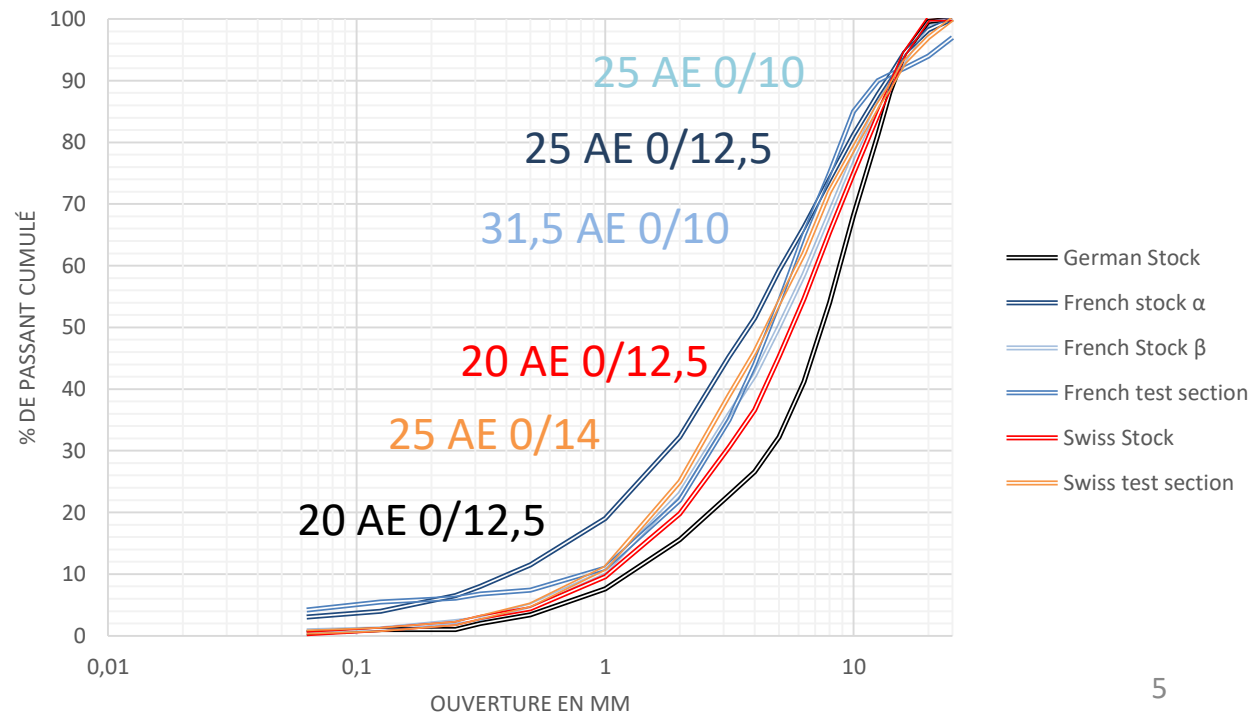
Materials description

- Two French stocks made up of : α (with PAH) and β (without PAH) :
 - Stock α (Lingenheld) with :
 - 298 mg/kg of PAH constituted in 2017 ;
 - Codification EN 13108-8 : 25 AE 0/10
 - Stock β (Lingenheld) with :
 - 8 mg/kg of PAH constituted in 2018 ;
 - Codification EN 13108-8 : 25 AE 0/12,5
- A German stock (20 AE 0/12.5)
- A Swiss stock (20 AE 0/12,5)
- The materials of the Swiss in situ test section (25 AE 0/14)
- The materials of the French in situ test section (31,5 AE 0/10)

Characteristics of RAP

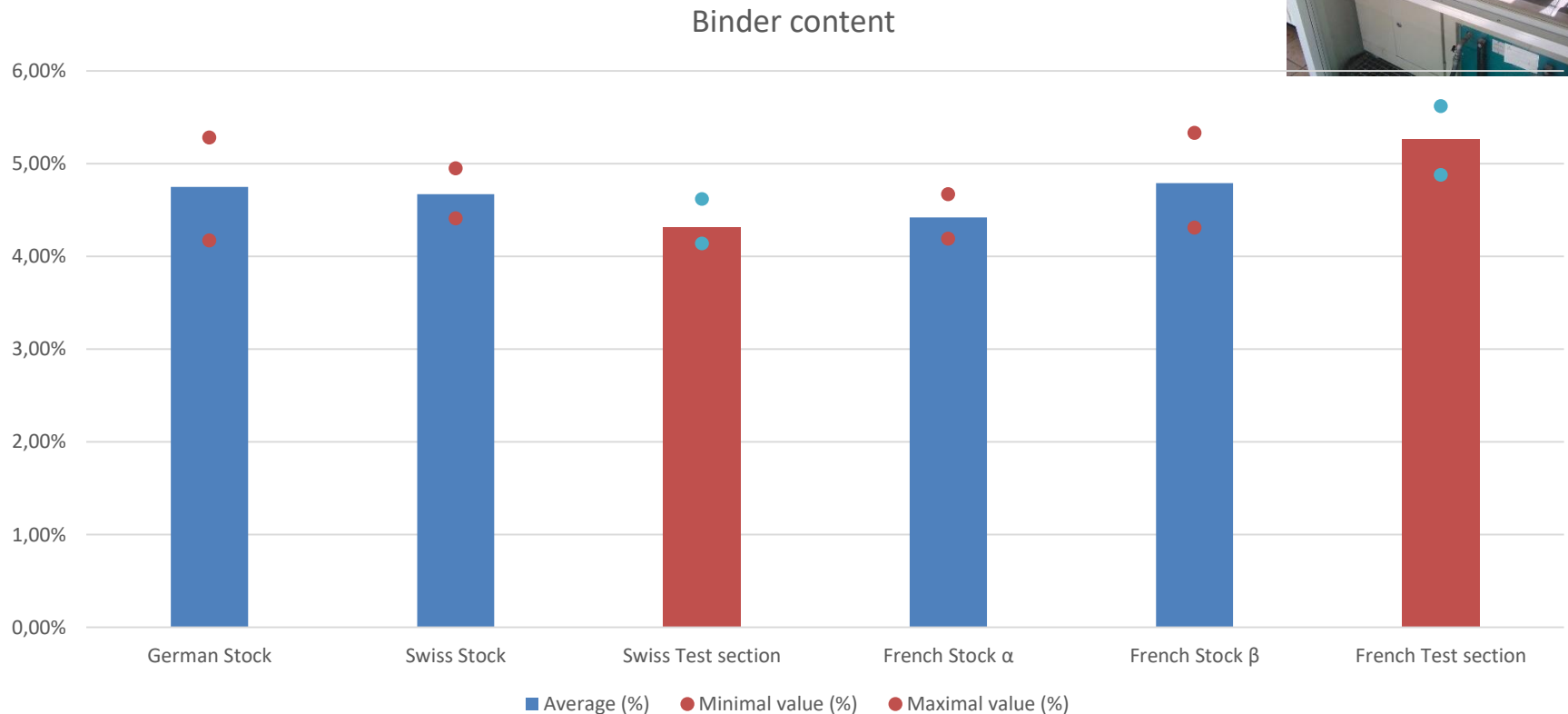
Granulometric curve of German, Swiss and French materials, obtained on asphalt aggregates

Distribution curve



Characteristics of RAP

Results of binder content / Classification



➔ **Codification TL2 to TL1... (Range min-max < 2 %)**

Characteristics of RAP

Maximum Density of RAP (EN 12697-5)

	German Stock	Swiss Stock	French Stock β	French Stock α
Average (Mg/m ³)	2,479	2,510	2,467	2,481
Range Δ	46 kg / m ³	4 kg/m ³	9 kg/m ³	11 kg/m ³
	< 50 kg/m ³			
Estimated density of aggregates	2,662 Mg/m ³	2,699 Mg/m ³	2,656 Mg/m ³	2,653 Mg/m ³



Characteristics of RAP

Foreign elements (EN 12697-42)

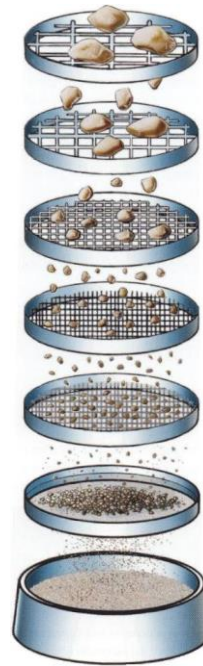
Category	Maximal value of foreign elements
F ₁	material contents of group 1 < 1% and material contents of group 2 < 0.1%
F ₅	material contents of group 1 < 5% and material contents of group 2 < 0.1%
F _{dec}	Content and nature of foreign materials declared

German Stock, Swiss stock, Swiss Test section & French Test section :
 no foreign elements detected in the stock

French Stock β : Presence of metal parts (0,2 %)



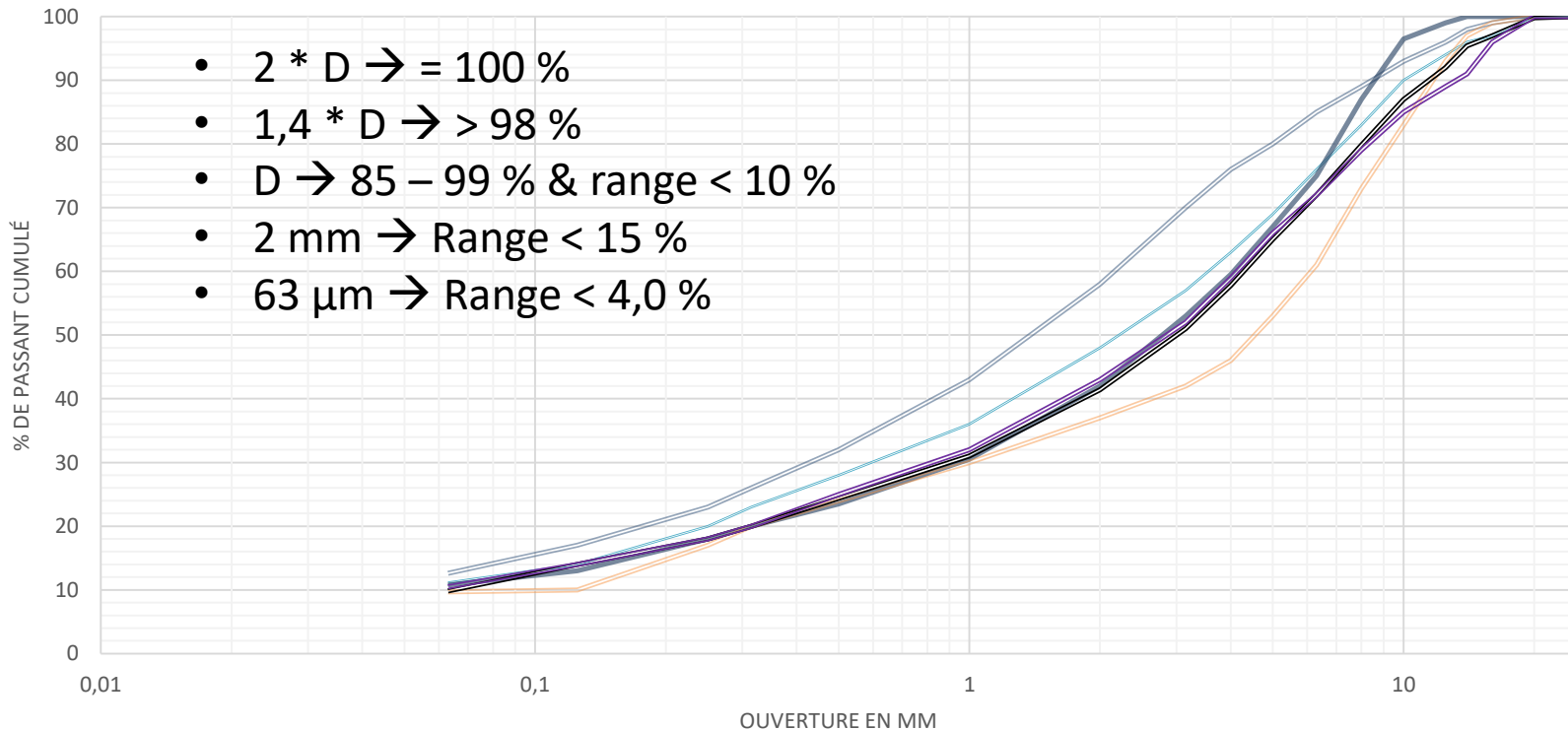
Codification F1



Characteristics of aggregates

Distribution Curve : EN 12697-2

Distribution curve after extraction



Codification G2 to G1

Characteristics of aggregates

Resistance to fragmentation : LA (EN 1097-2)

	German Stock	Swiss Stock	French Stock β	French Stock α	
Granularity 4/6,3	18,5	16,9	18,5	-	→ 18
Granularity 6,3/10	18,2	17,0	17,8	17,5	→ 18
Granularity 10/14	17,0	16,4	17,4	16,5	→ 17

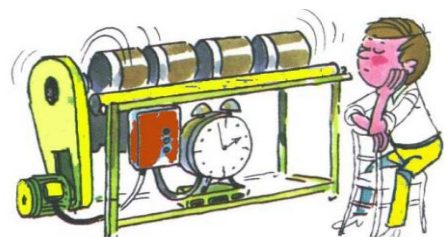


**Codification R1
(LA < 25)**

Characteristics of aggregates

Resistance to wear : MDE (EN 1097-1)

	German Stock	Swiss Stock	French Stock β	French Stock α	
Granularity 4/6,3	12,2	8,7	11,7	-	➔ 11
Granularity 6,3/10	9,2	7,7	8,6	9,1	➔ 9
Granularity 10/14	7,9	7,6	8,1	9,9	➔ 8



Codification R1

Characteristics of aggregates

Angularity : Ang (EN 933-5)

		German Stock	French Stock	Swiss Stock
Granularity 4/8	Crushed or broken / Totally rounded	86 % / 6 %	74 % / 9 %	83 % / 8 %
	Category	Ang3 - C_{50/10}	Ang3 - C_{50/10}	Ang3 - C_{50/10}
Granularity 8/14	Crushed or broken / Totally rounded	90 % / 2 %	86 % / 3 %	78 % / 7 %
	Category	Ang2 - C_{90/3}	Ang3 - C_{50/10}	Ang3 - C_{50/10}



Codification R3 (< Ang 1)

Characteristics of aggregates

Flakiness Index (EN 933-3)

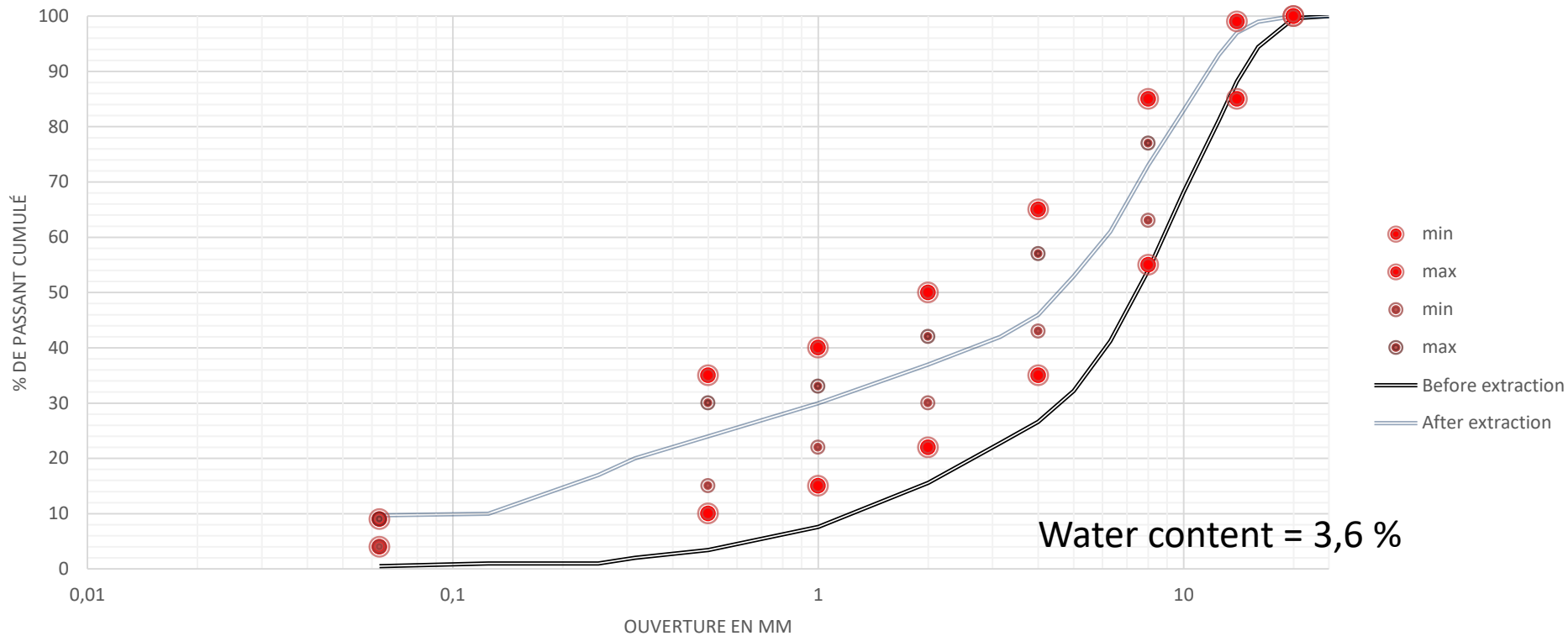
	German Stock	Swiss Stock	French Stock β	French Stock α
FI (%)	8,1 %	11,9 %	8,8 %	9,9 %



Characteristics of materials

- Distribution curve of german materials before / after extraction of bitumen (EN 13285)

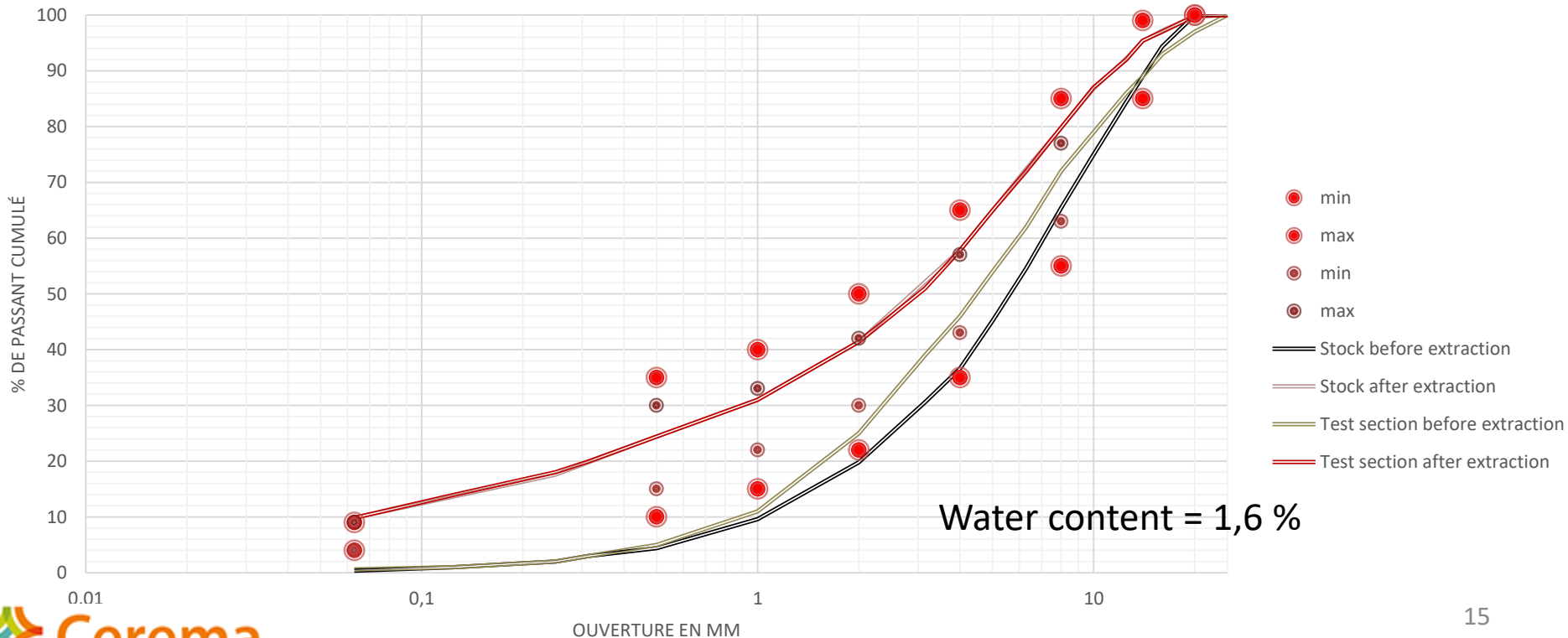
Granulométrie - stocks Allemagne



Characteristics of materials

- Distribution curve of swiss materials before / after extraction of bitumen (EN 13285)

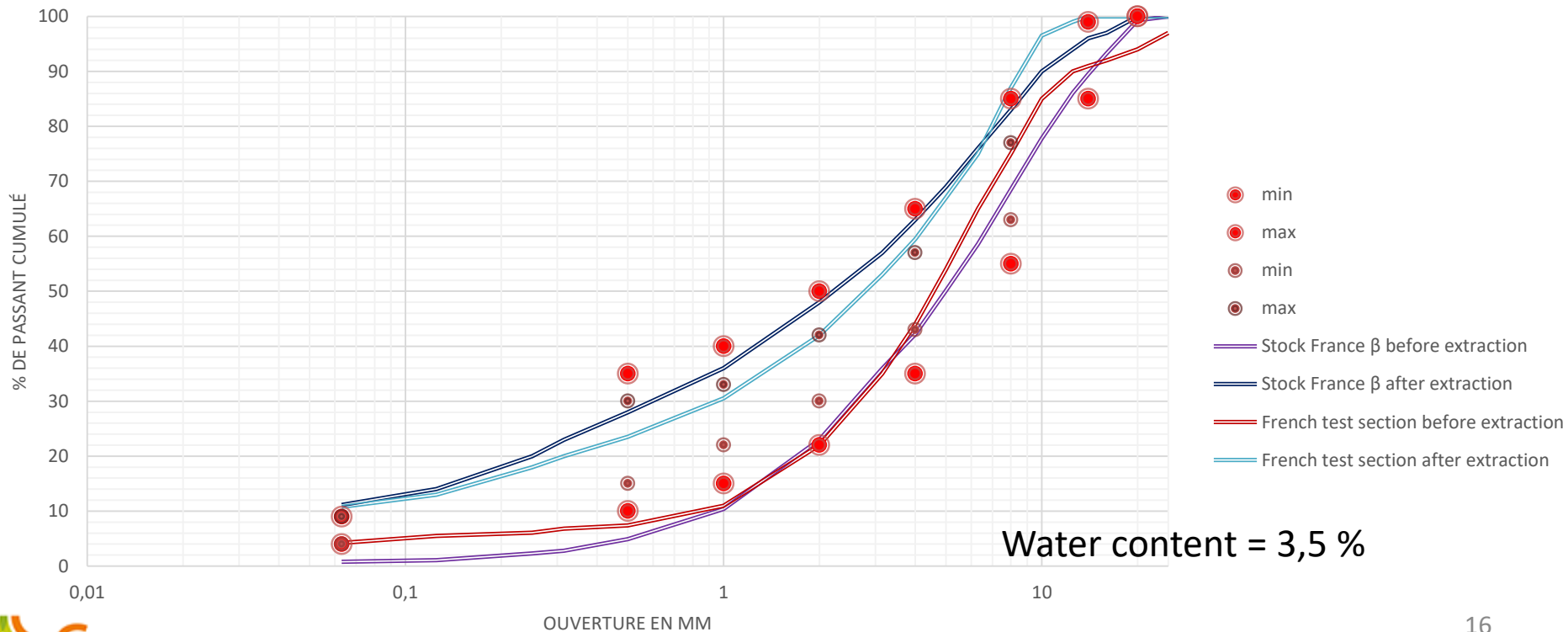
Distribution curve – Swiss stock & test section



Characteristics of materials

- Distribution curve of french materials before / after extraction of bitumen (EN 13285)

Distribution curve – French stock and test section



Conclusions

- Relatively homogeneous materials (intrinsic characteristics)
→ silico-limestone alluviums
- Materials more or less graded depending on stocks
- Extracted aggregates rather located at the top of the UGM standard spindle (EN 13285)



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Characterization of extracted bitumen

Hartmut Herb



Hochschule Karlsruhe
Technik und Wirtschaft
UNIVERSITY OF APPLIED SCIENCES

Determination and Methods:

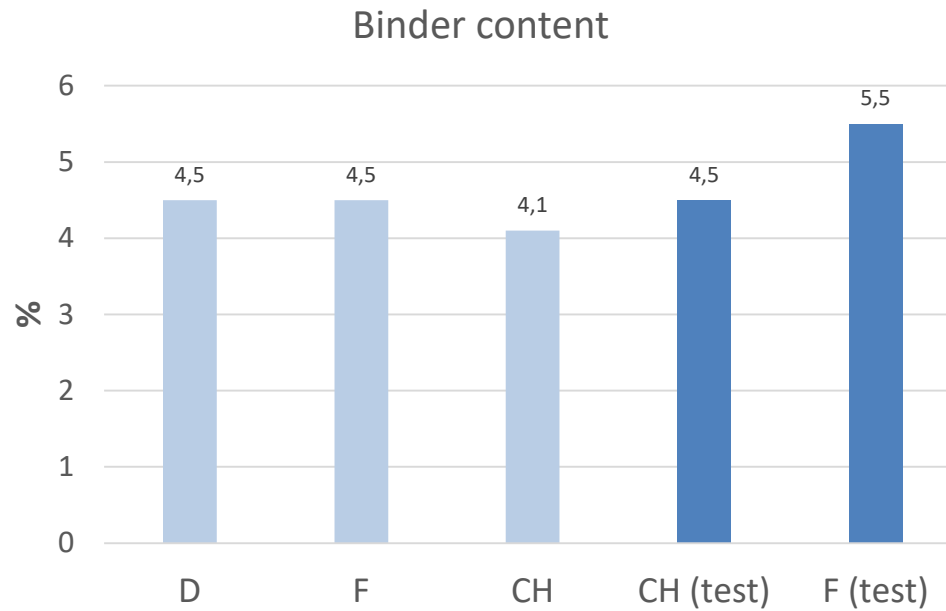
Physico-mechanical:

- Binder content
- Softening point
- Needle-Penetration
- Elastic recovery
- Density
- DSR
- BBR

Chemical:

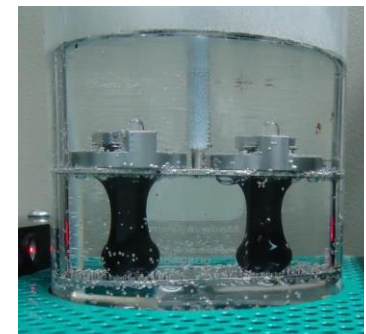
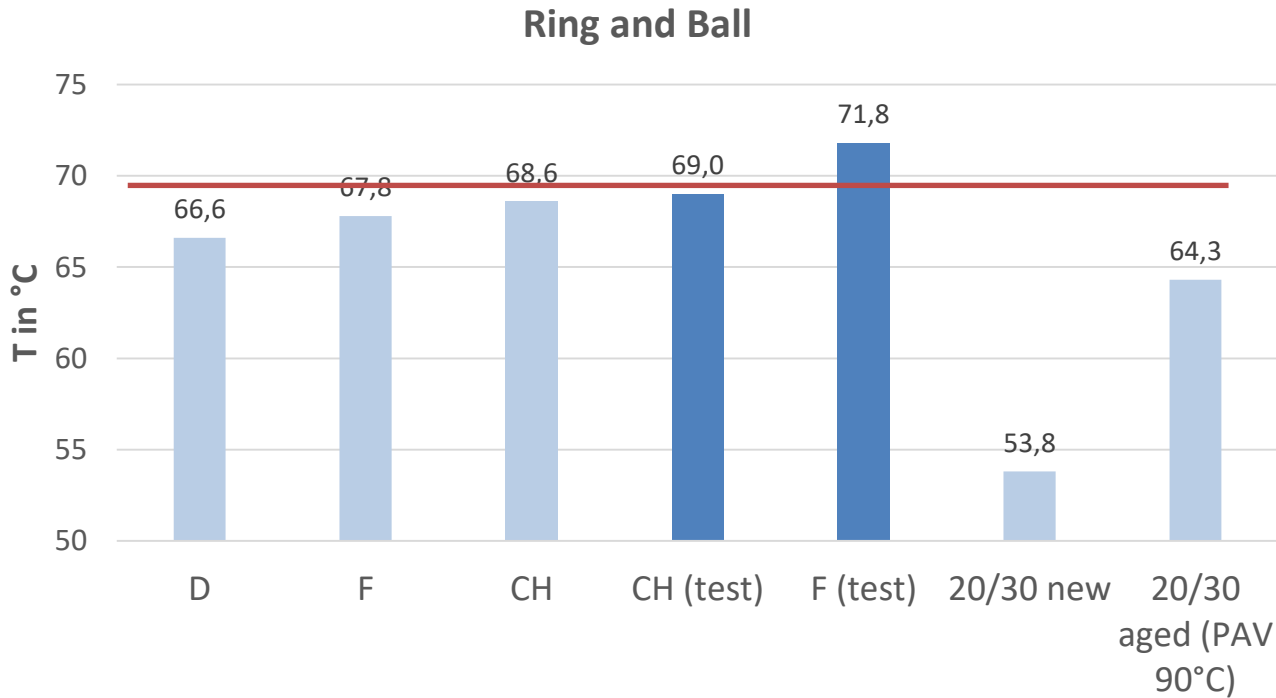
- Molecular weight
- Indices of Oxidation
- SARA

Binder content (EN 12697-3)



Ring and Ball (EN 1427)

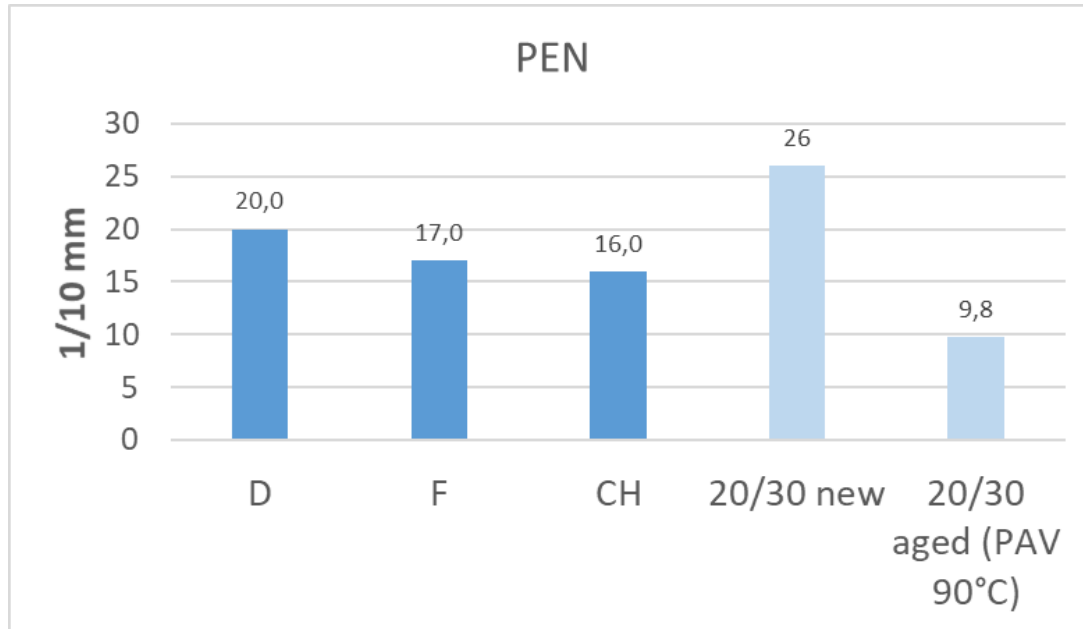
Determination of the softening point



Regulation: limit 70°C (average), 77°C (maximum)

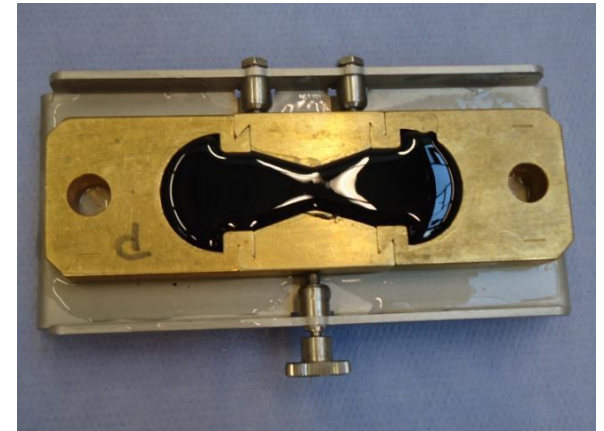
PEN (EN 1426)

Determination of needle penetration

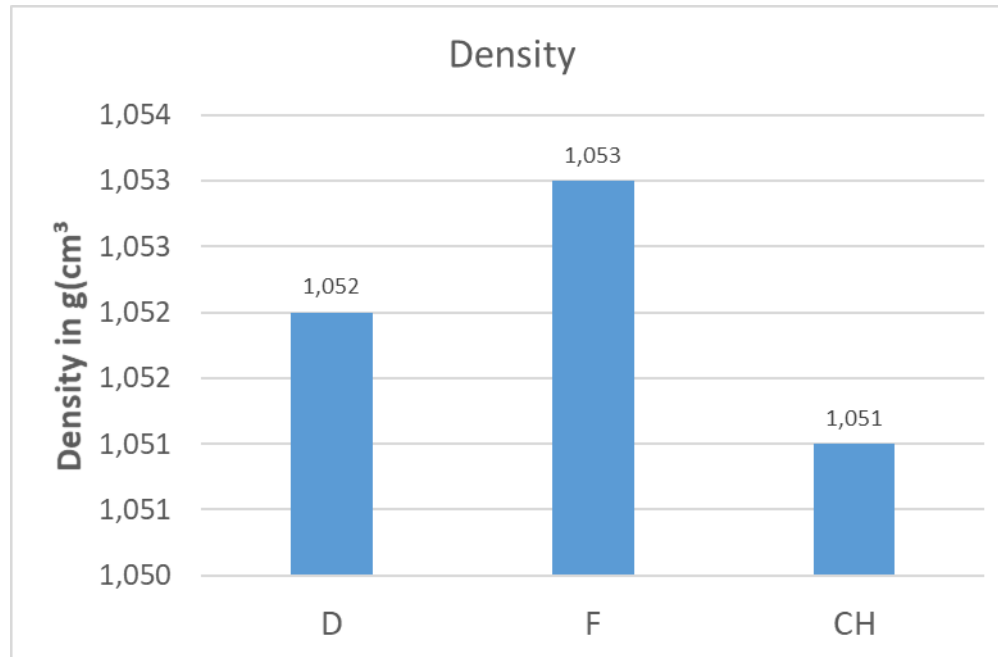


Elastic recovery test (EN 13398)

results:
no elastic recovery of each Bitumen

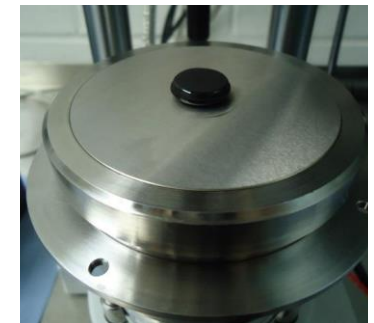
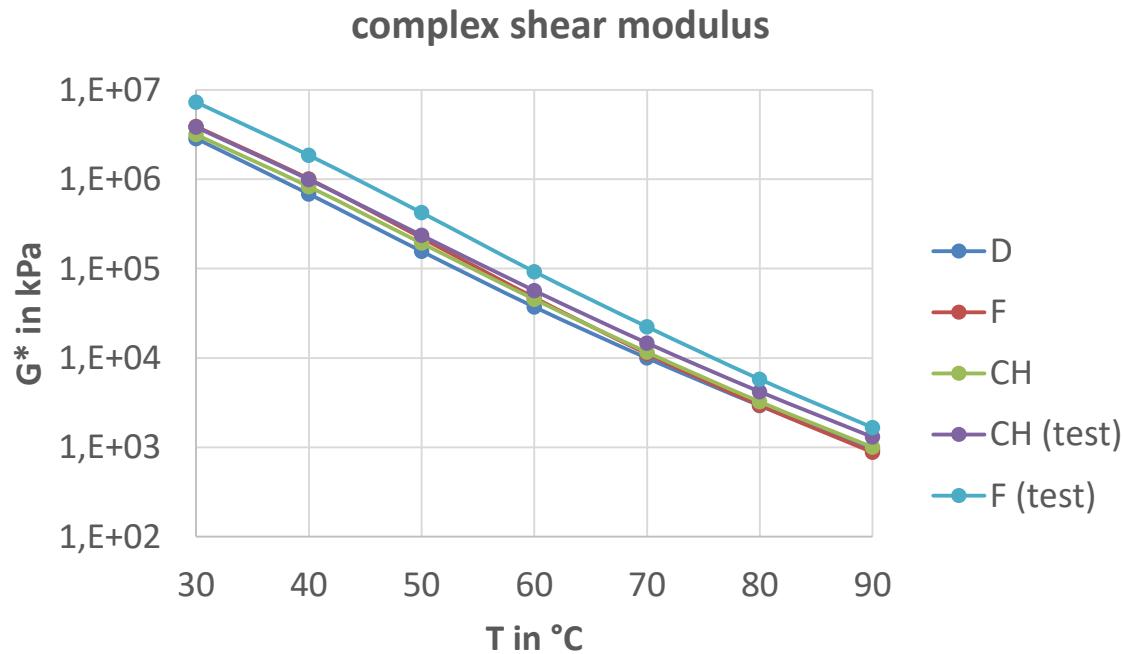


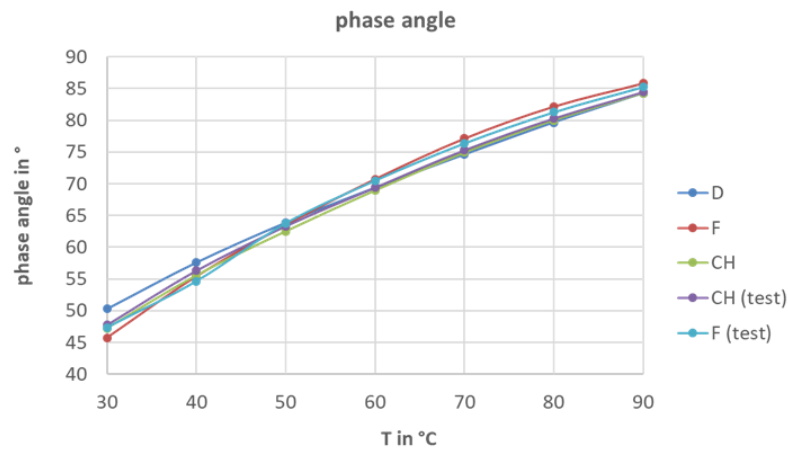
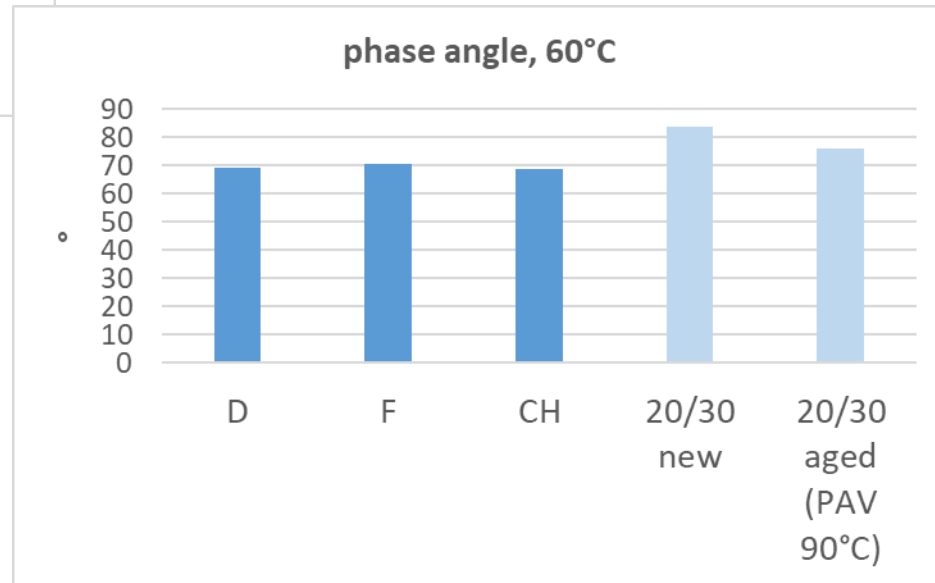
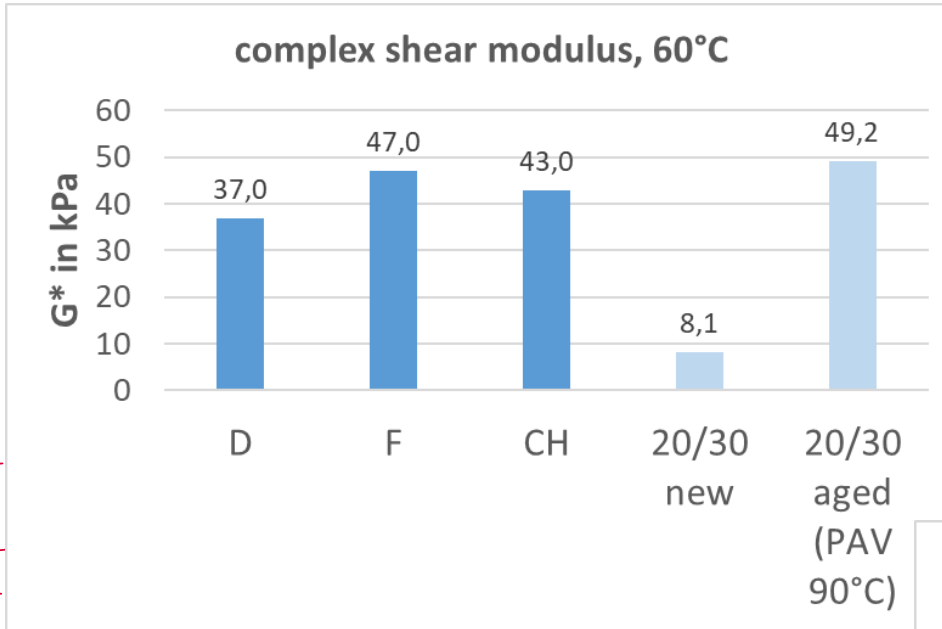
Density (EN 15326)



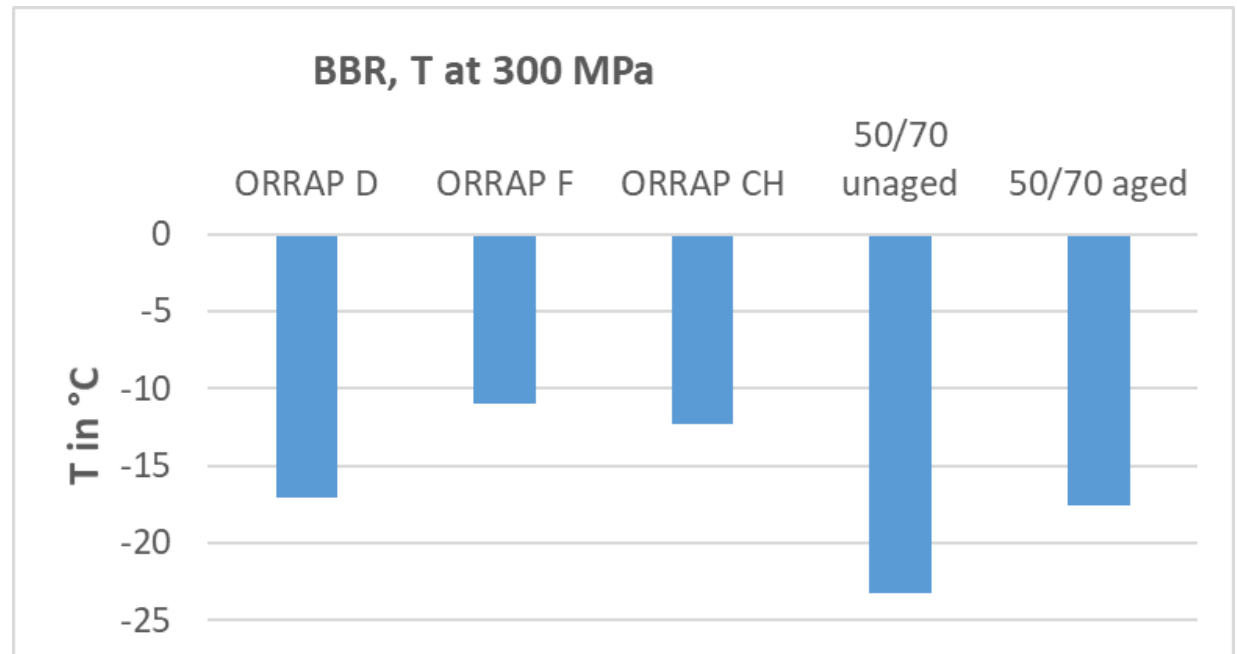
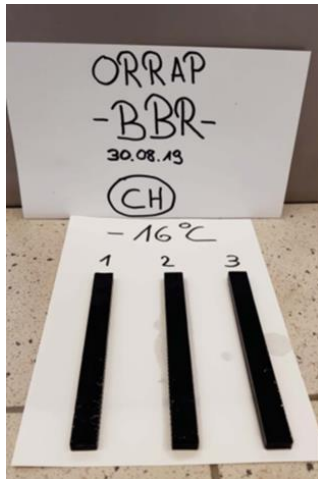
Dynamic shear rheometer (DSR, EN 14770)

Determination of complex shear modulus and phase angle (d=25mm, s=1mm)





Low temperature performance by Bending Beam Rheometer, BBR



Molecular weight

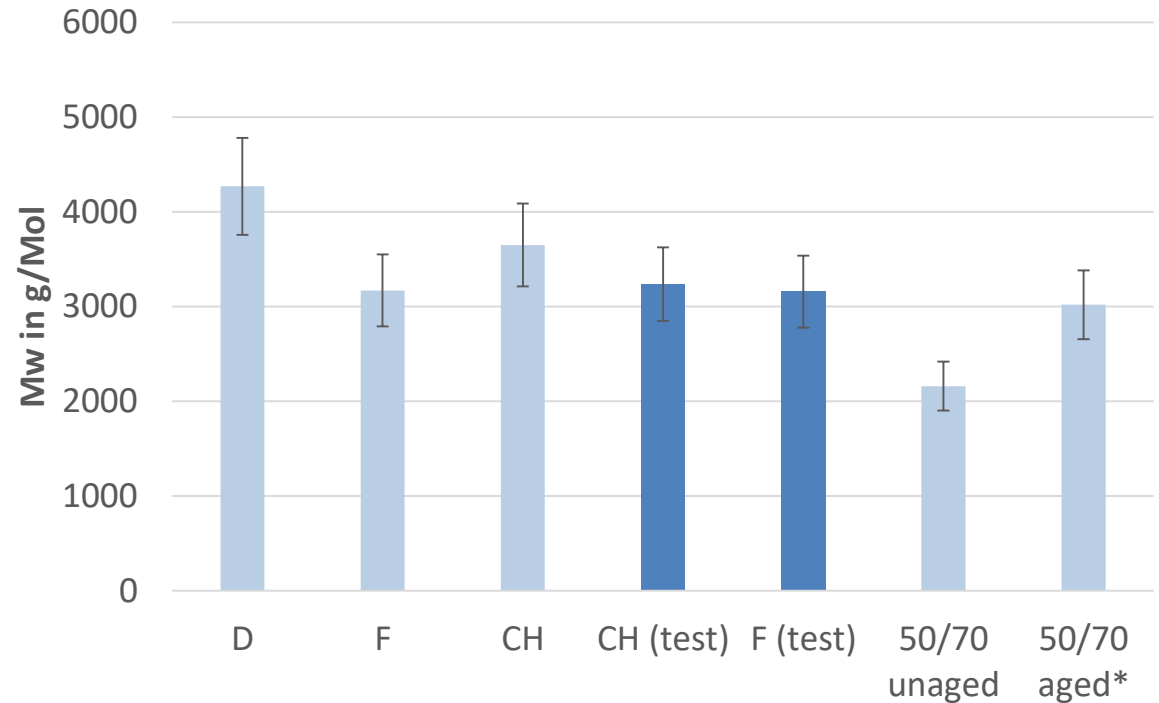
by gel permeation chromatography, GPC

Method of liquid chromatography

separation of compounds by hydrodynamic volume, i.e. by size and shape in a porous system

solid phase: SDV
solvent: THF
detector: RI
calibration: polystyrene

molecular weight of extracted bitumen (RI)



SARA-analysis

Two step method:

precipitation of **Asphaltenes** (iso-octane insolubles)
and liquid-chromatographic separation of **Maltenes**

information:

quantitative separation of bitumen in

Saturates

Aromatics (mono-, di-, poly-)

Resins

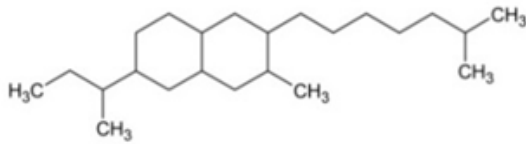
Asphaltenes

} **Maltenes**

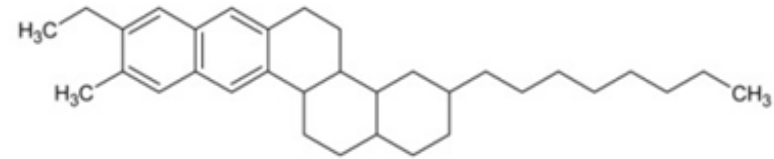


Schematic structures of SARA-compounds

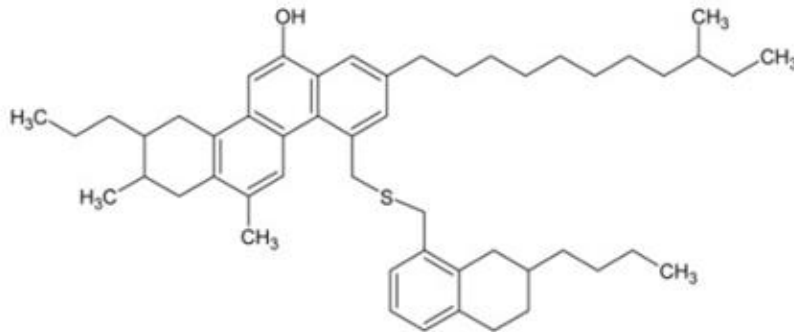
Saturates



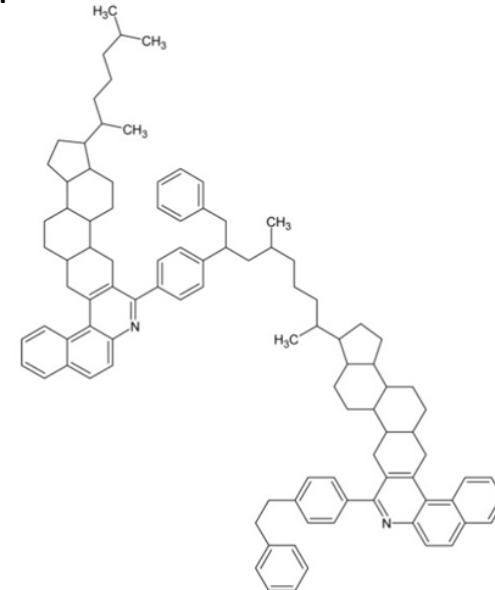
Aromatics



Resins



Asphaltenes



procedure of Maltene separation (Sebor et al.)

Column material:

alumina/silicagel

Solvents:

Saturates: Iso-octane

Mono-aromatics: Iso-octane/Toluene 19:1

Di-aromatics: Iso-octane/Toluene 17:1

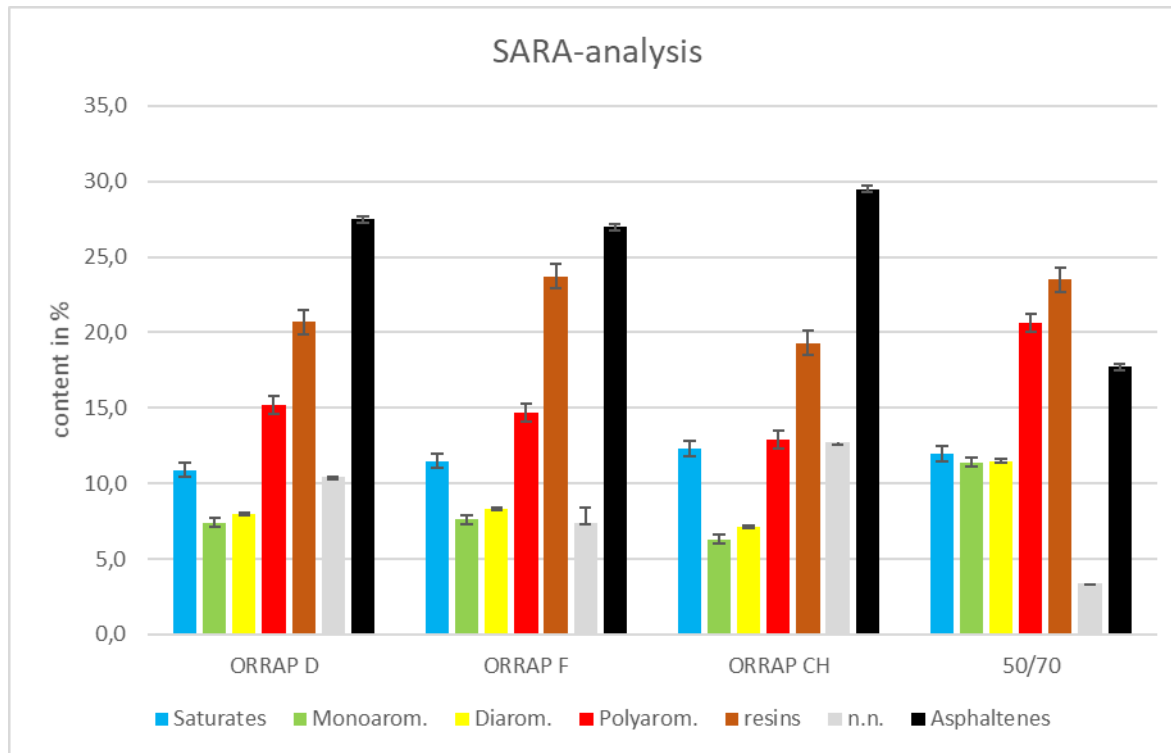
Poly-aromatics: Toluene

Resins: Methanol/Diethylether/Toluene 3:1:1

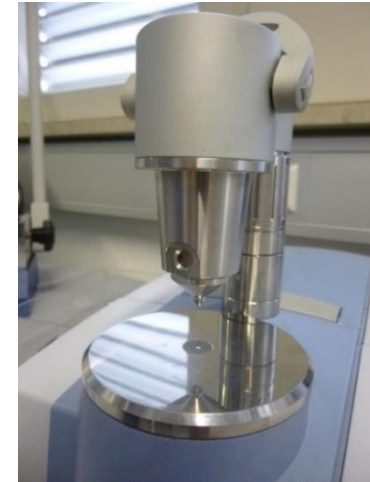
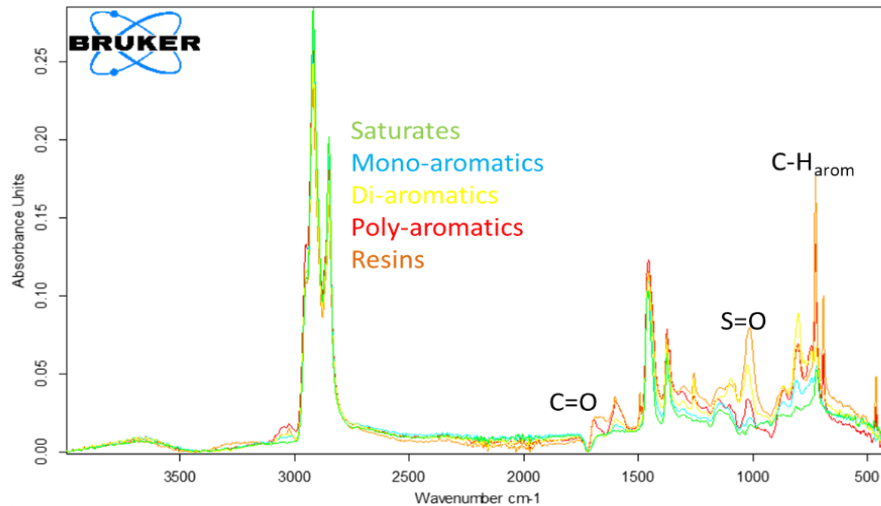
increasing polarity

Evaluation:

Determination of mass after solvent evaporation and drying



Oxidation state by FTIR-ATR-technique

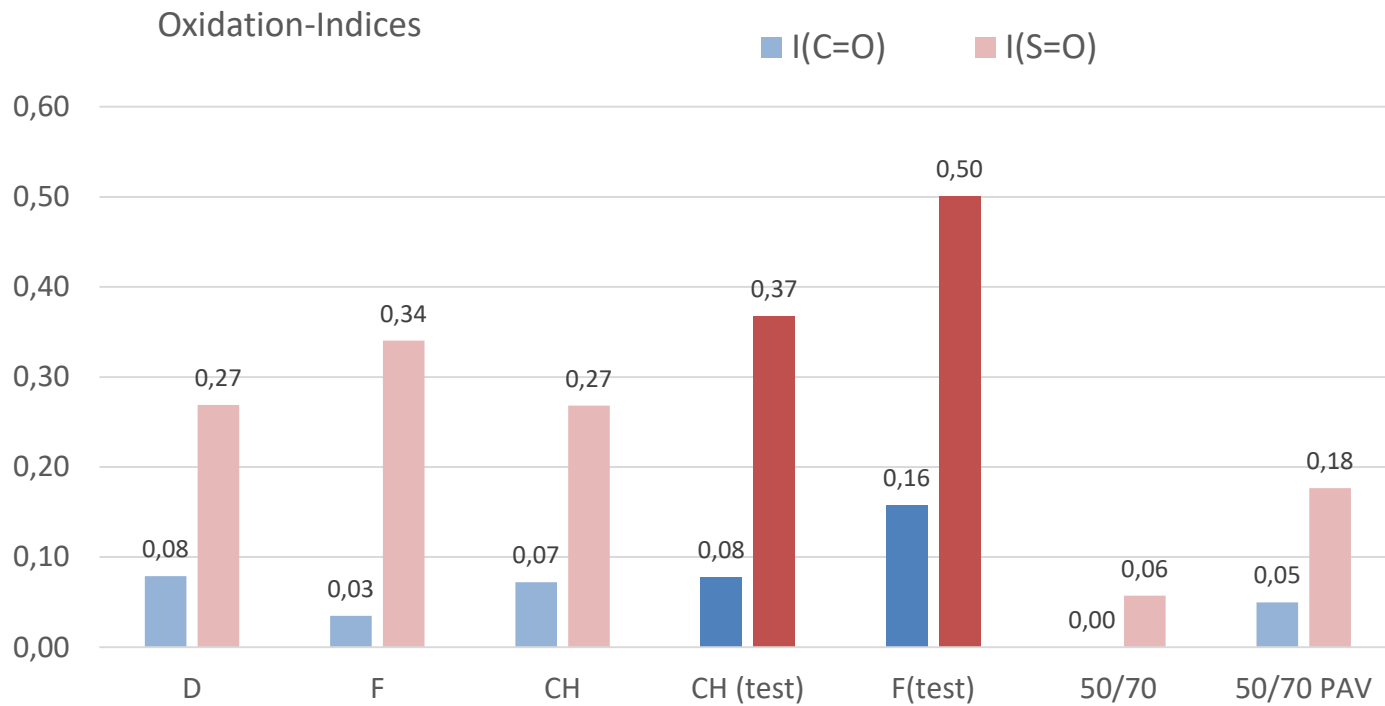


Quantification by Carbonyl- and Sulfoxid-Indices

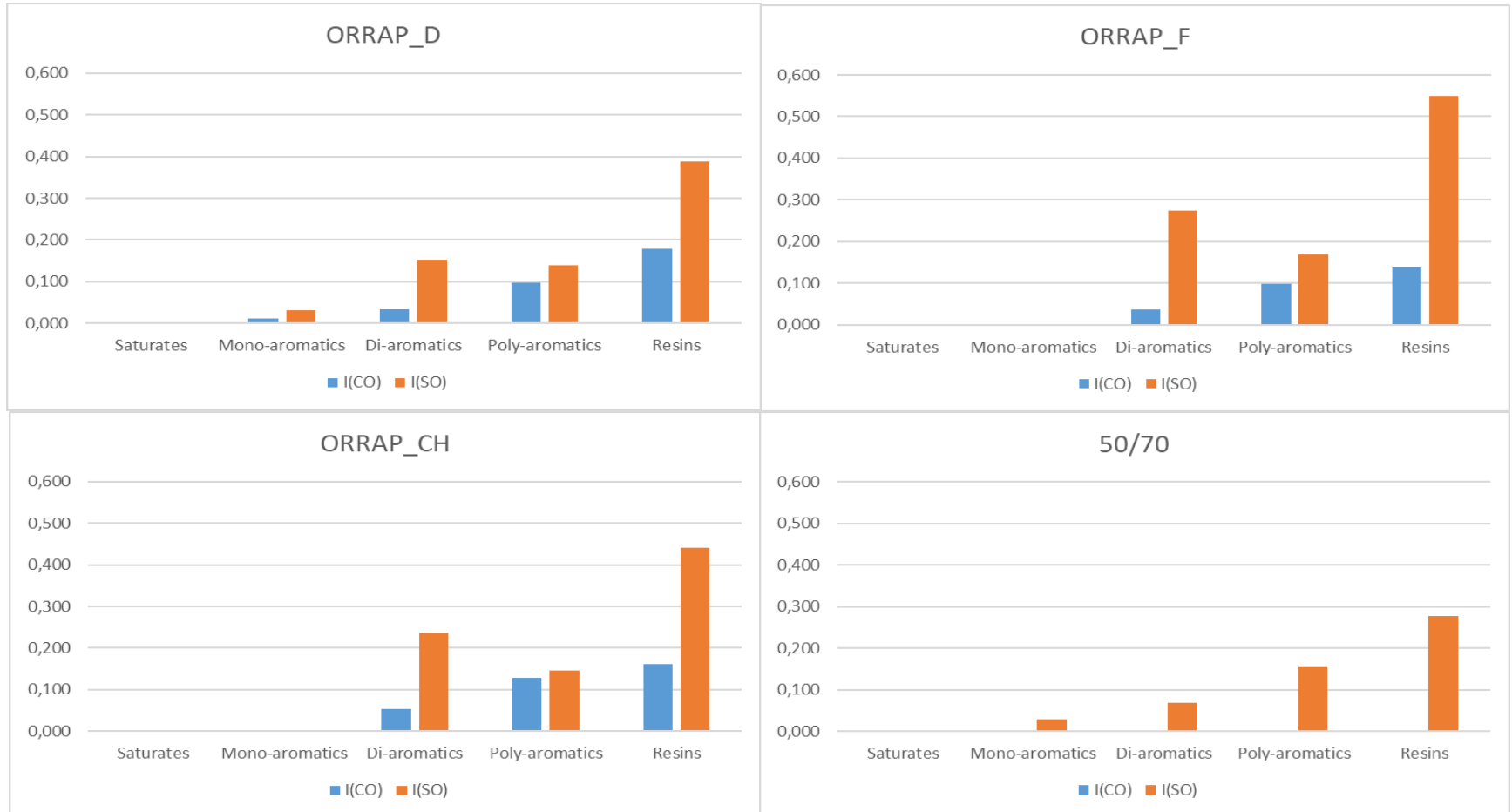
$$I_{CO} = \frac{\text{Area of the } C=O \text{ peak around } 1700 \text{ cm}^{-1}}{\text{Area of the } CH \text{ peak around } 1455 \text{ cm}^{-1} + \text{Area of the } CH \text{ peak around } 1376 \text{ cm}^{-1}}$$

$$I_{SO} = \frac{\text{Area of the } S=O \text{ peak around } 1030 \text{ cm}^{-1}}{\text{Area of the } CH \text{ peak around } 1455 \text{ cm}^{-1} + \text{Area of the } CH \text{ peak around } 1376 \text{ cm}^{-1}}$$

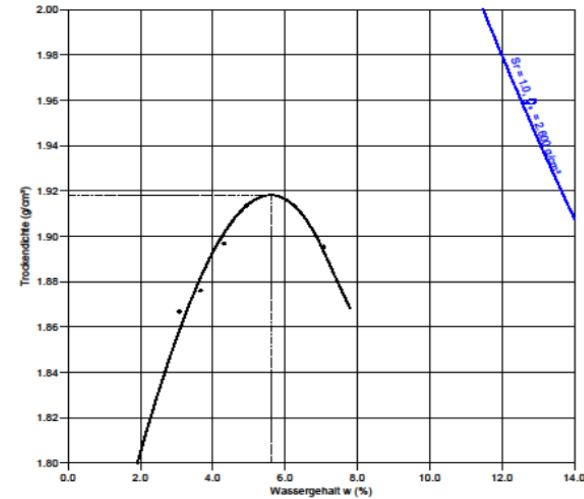
Extracted bitumen



Separated Maltenes



Modified Proctor Test (DIN 18127)



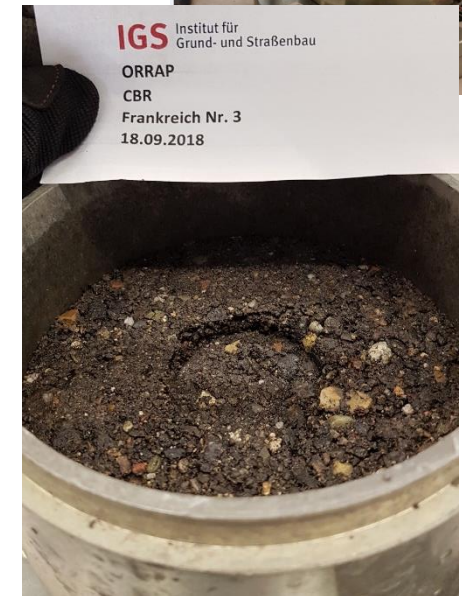
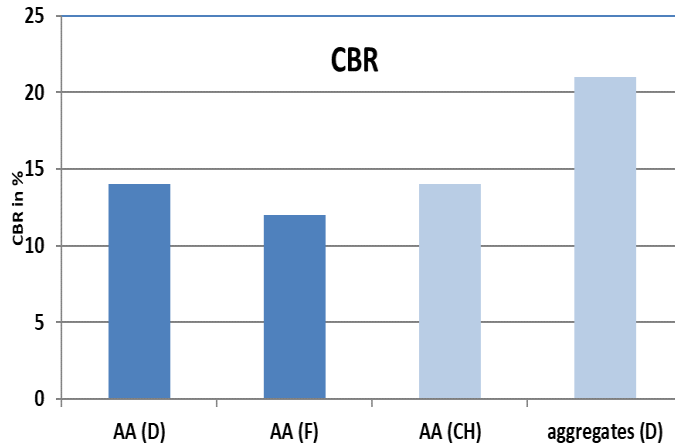
		D	F	CH
max. density	g/cm ³	1,92	1,79	1,81
opt. moisture content	%	5,6	5,5	4,3

CBR (California Bearing Ratio) (EN 13286-47)

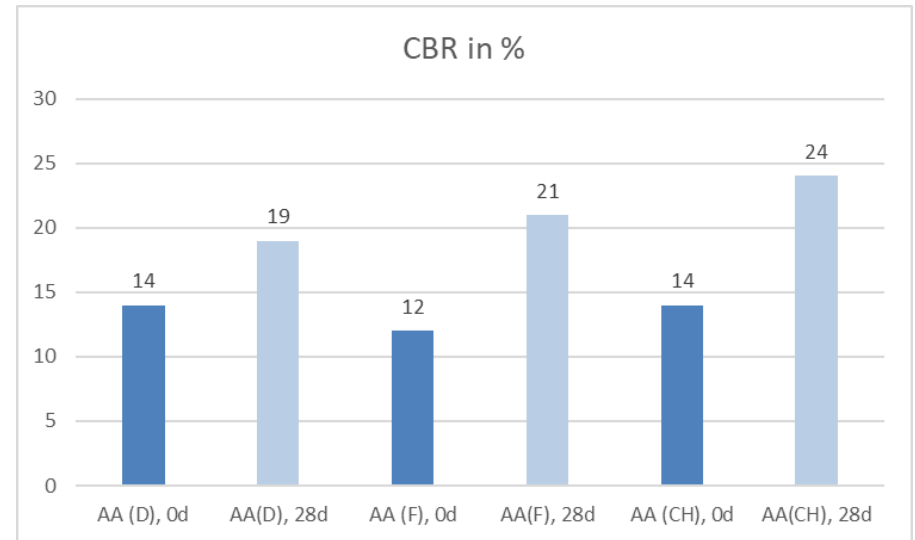
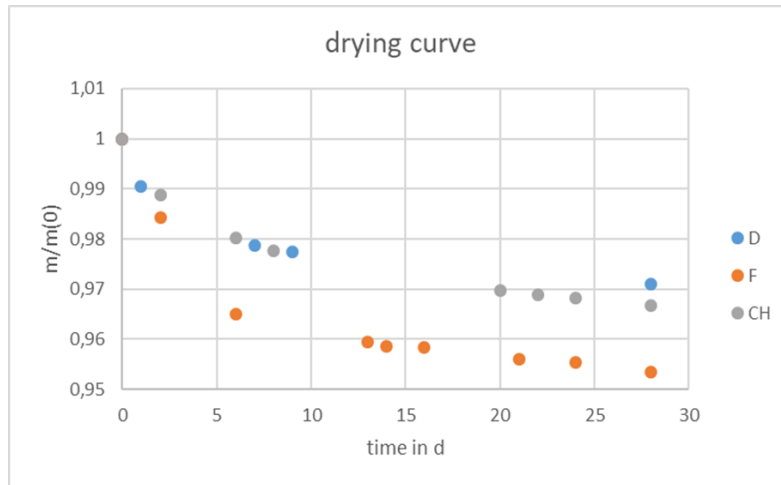
→ measuring the pressure required to penetrate soil or aggregate with a plunger

AA compacted by **modified Proctor test**

value compared to certified limestone in %

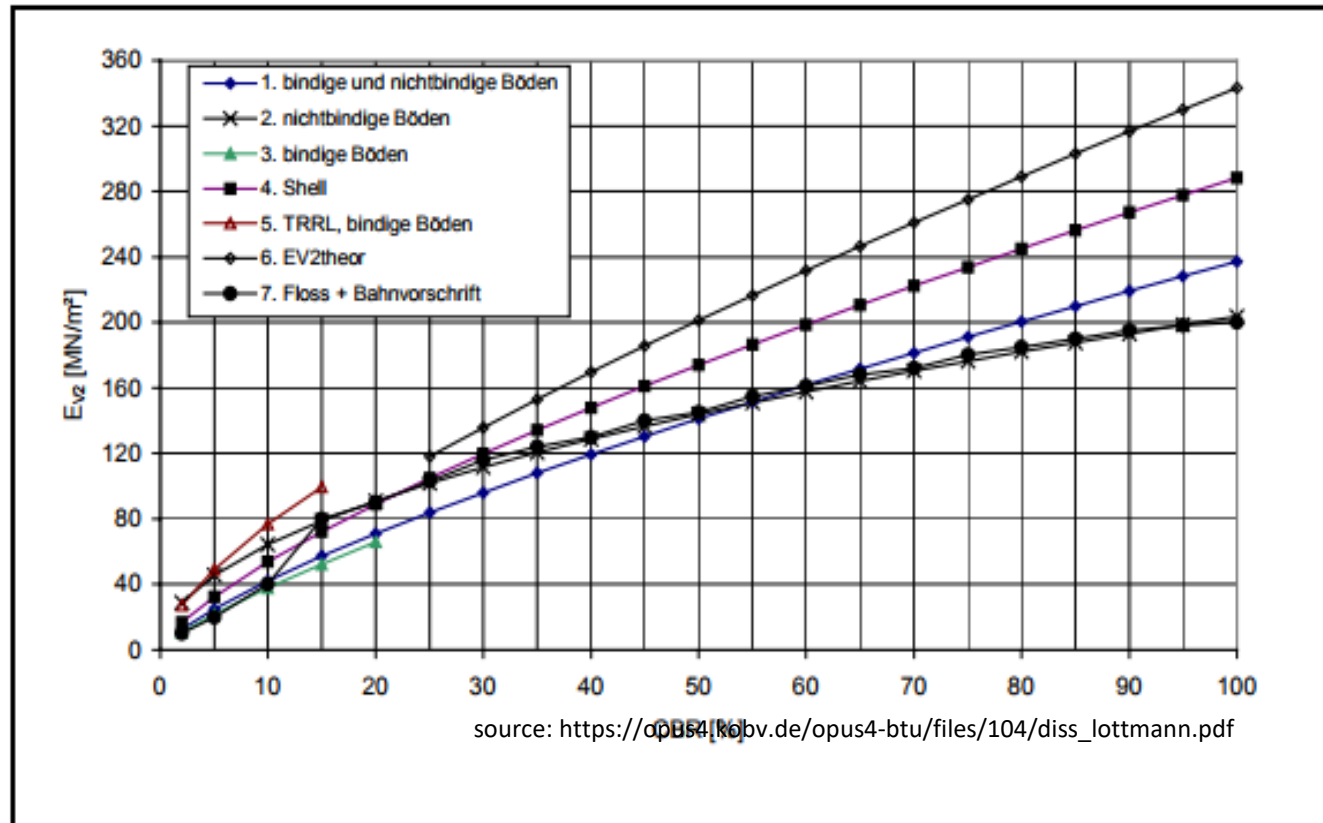


Effect of drying



→ increase after 28 d for each AA

Correlation between CBR and E_{V2} -modulus of deformation



- max. E_{V2} -values for AA: ca. **75 N/mm²**
- verification in situ by plate-loading test



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PART III

Mechanical Behaviour (GSR & ITS tests)

Arnaud Feeser, Cerema



Mechanical behaviour

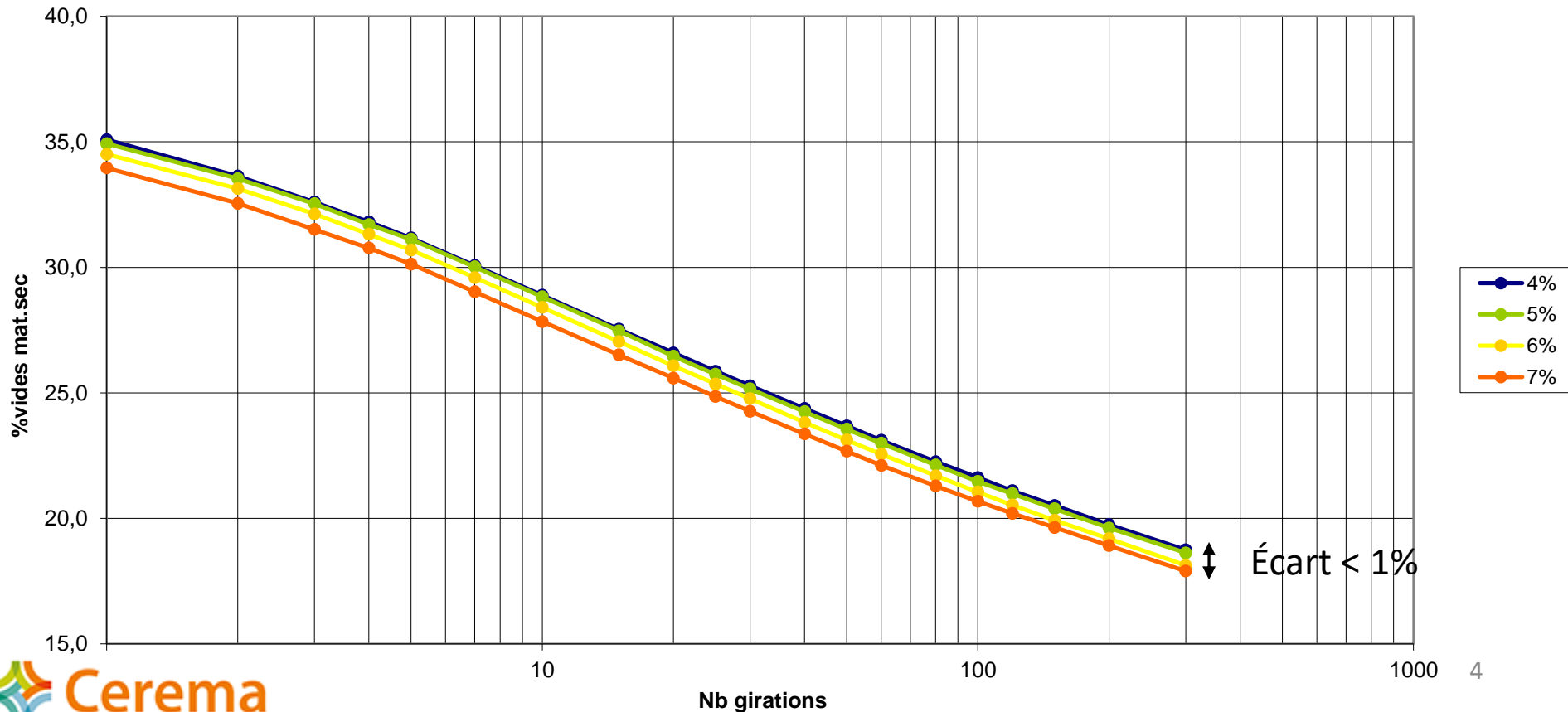
Assessment of the impact of **moisture content** on compactibility (PCG) – EN 12697-31



- Protocol
 - 300 gyrations
 - 4 water contents (4 to 7 %)
 - French stock α

Mechanical behaviour

Assessment of the impact of **moisture content** on compactibility (PCG) – EN 12697-31



Mechanical behaviour

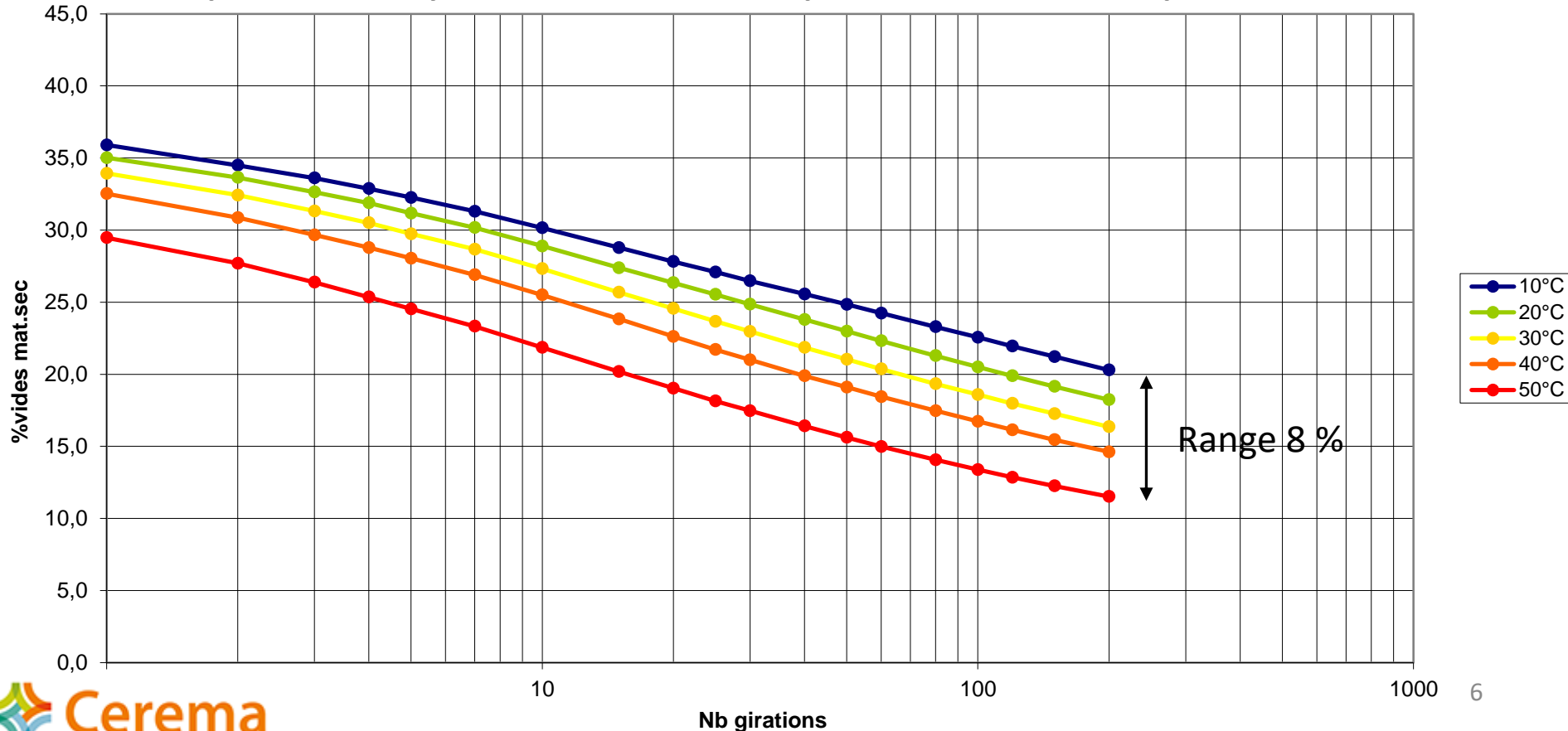
Assessment of the impact of **temperature** on compactibility (PCG) – EN 12697-31



- Protocol
 - 200 gyrations
 - 5 temperatures (10 to 50 °C)
 - German and French β stocks
 - Compaction to MPO water content

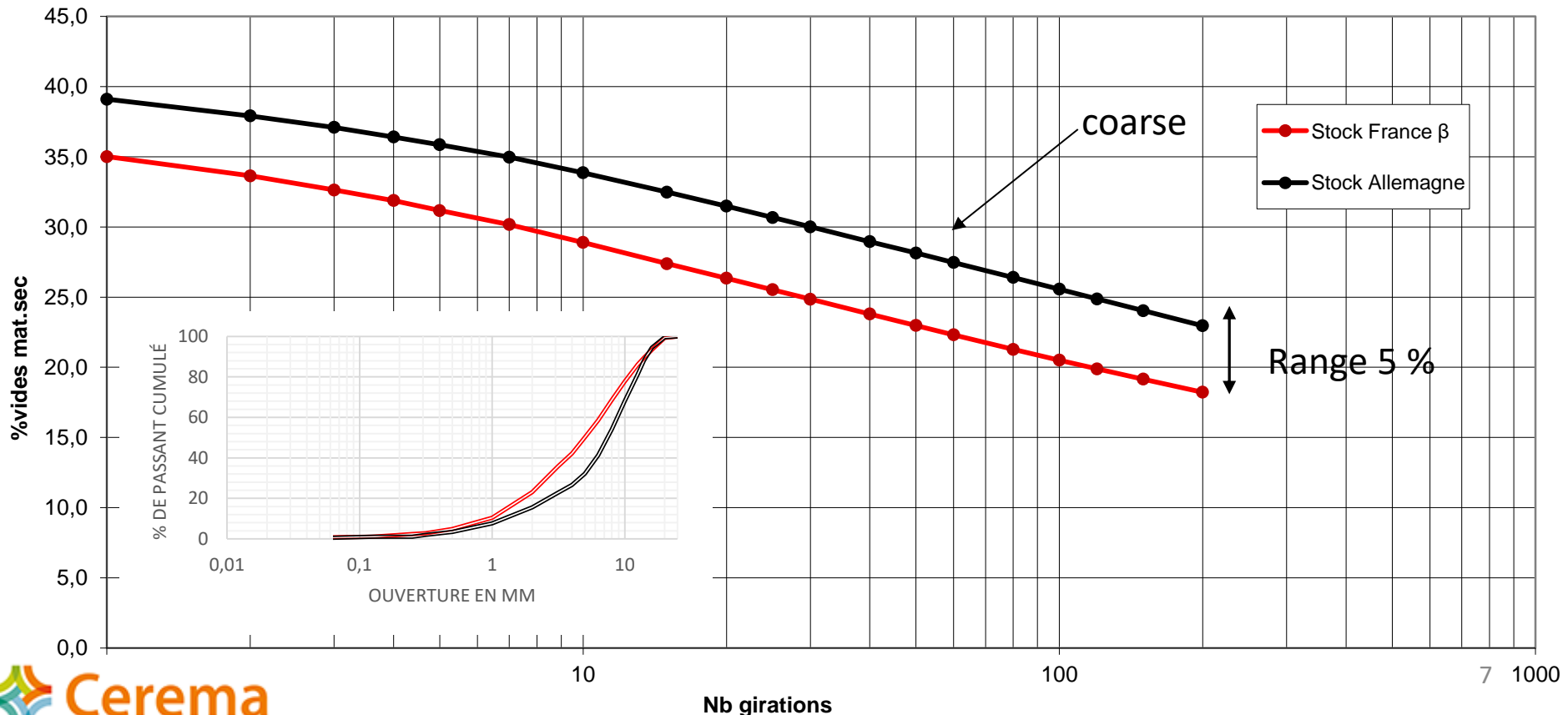
Mechanical behaviour

Assessment of the impact of **temperature** on compactibility (PCG) – Example for France β Stock



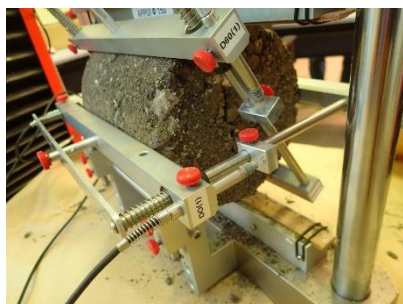
Mechanical behaviour

Evaluation of the impact of **grain size** on compactibility (PCG) at room temperature (20 °C)

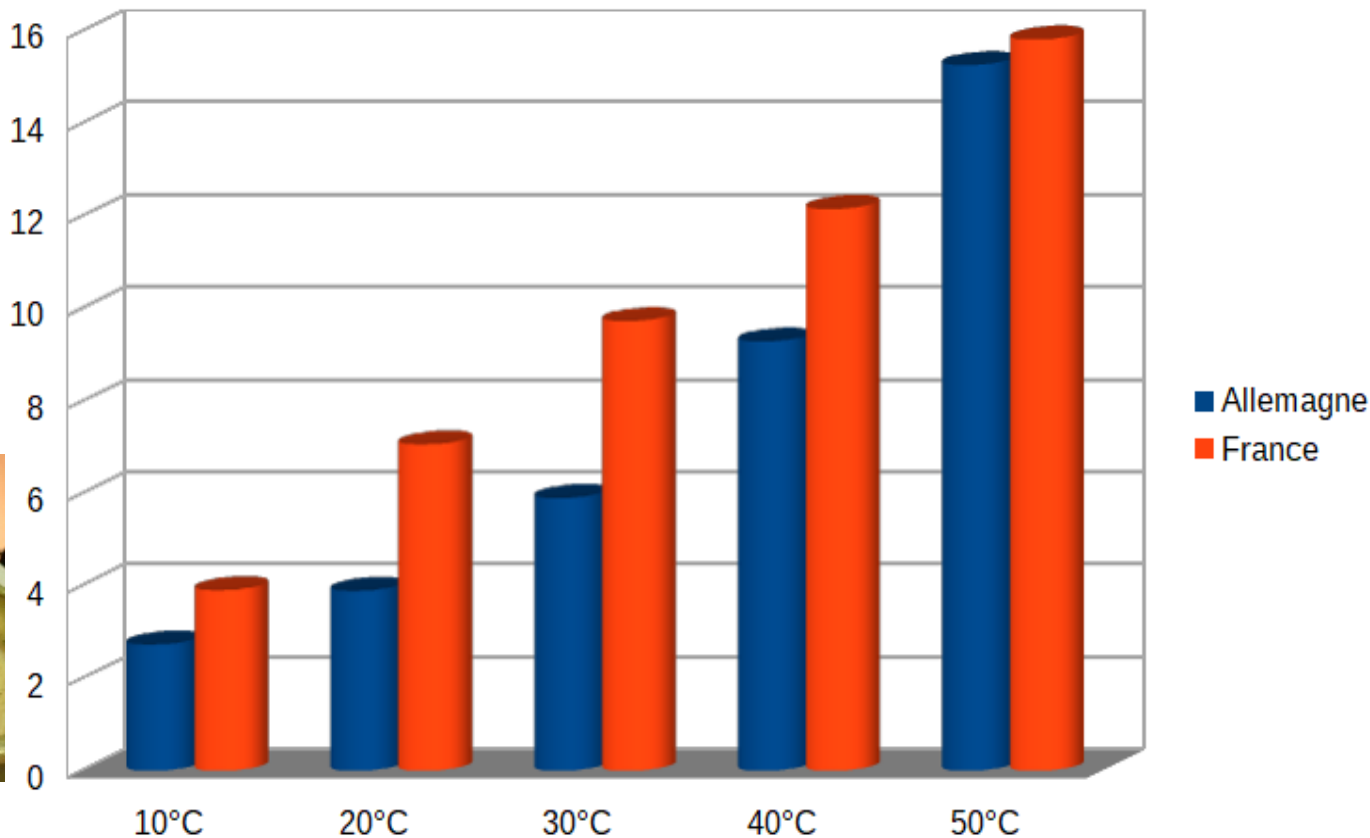


Mechanical behaviour

Evaluation of the **impact of temperature compaction** on ITS at 20 °C – Stocks : Germany & France

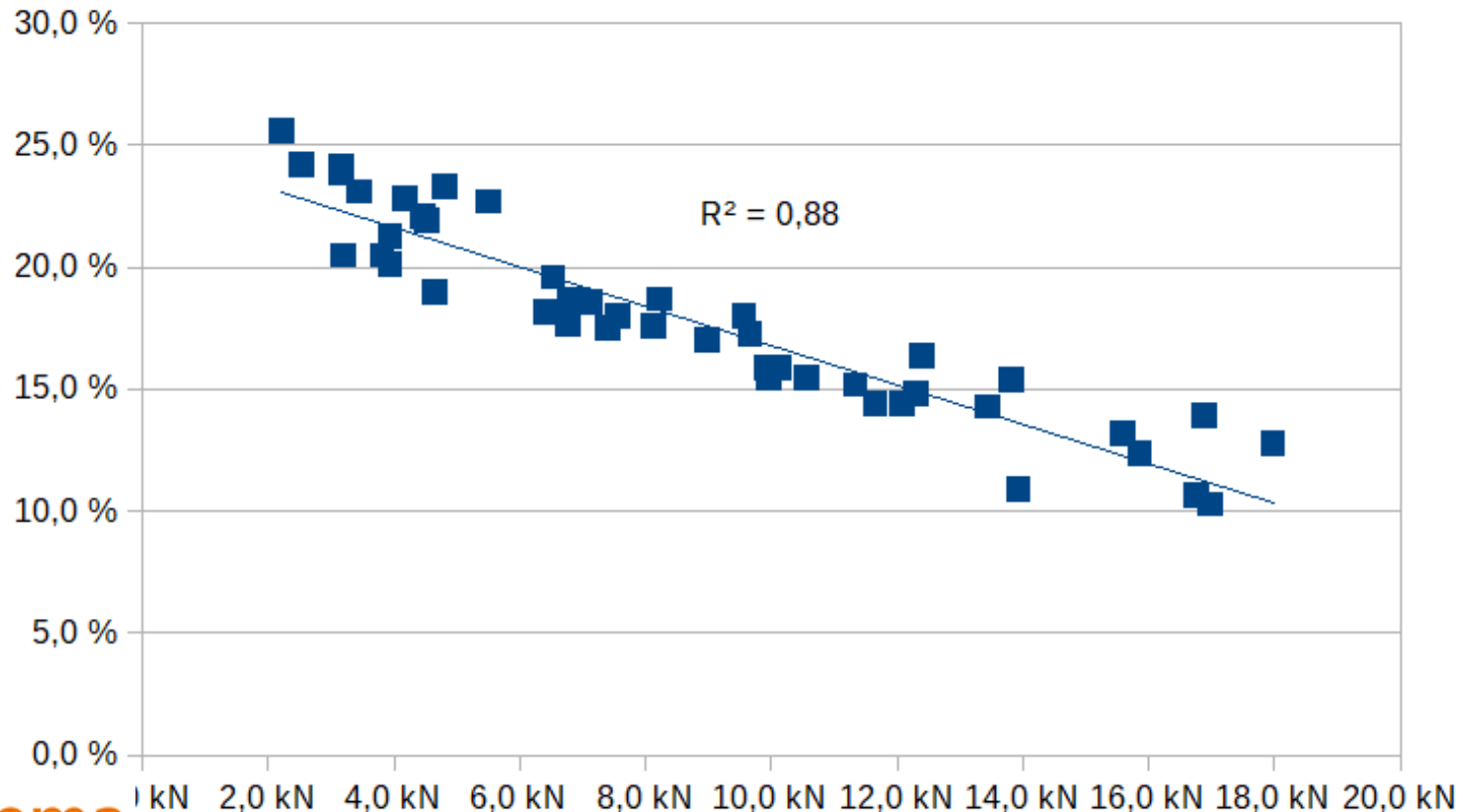


kN



Mechanical behaviour

Evaluation of the **impact of voids** on ITS at 20 °C –
Stocks : Germany & France



Conclusions

- Strong impact of the compaction temperature on compactness the material and its strength
- Relatively low impact of water content on material compaction
- (Normal) impact of the void content on the strength of the material



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Triaxial tests

Laura Gaillard

laura.gaillard6@gmail.com

Sampling

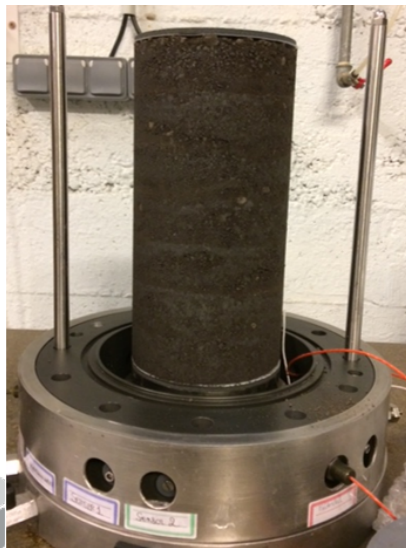
Mix material + water



Compaction of sample

Device at INSA
 Vibrating hammer
 Sample 300 mm x 150 mm

Device at IFSTTAR
 Vibrocompression
 Sample 320 mm x 160 mm



RAP

RAP after binder extraction

Rupture tests

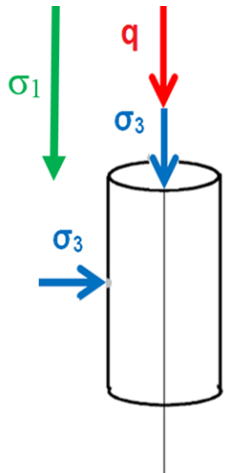
Studied materials from laboratory of France

RAP F α : French stock α with PAH

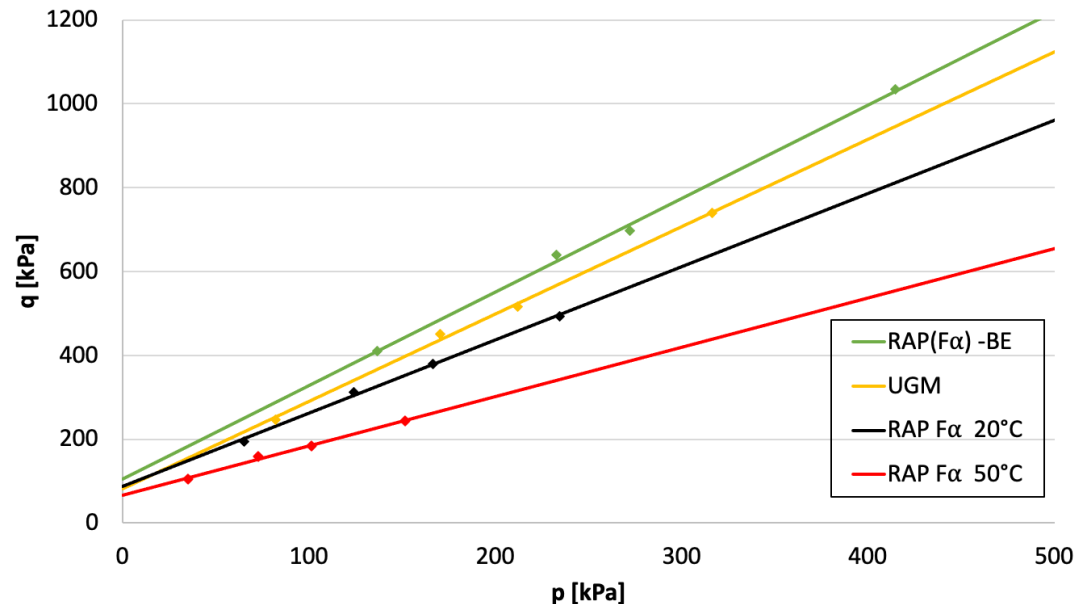
RAP_{F α} -BE : RAP F α after binder extraction

UGM : UGM from Rhine Region

	Friction angle ϕ [°]	Cohesion c [kPa]	Water content w [%]	Dry density γ_d [Mg/m ³]	Void content
RAP Fα 20°C	42.58	45.90	3.9	1.96	0.27
RAP Fα 50°C	34.13	28.60	3.9	1.96	0.27
RAP_{Fα}-BE	54.39	66,21	3.3	2.14	0.24
UGM	50.69	47.42	3.9	1.96	0.35



Rupture lines

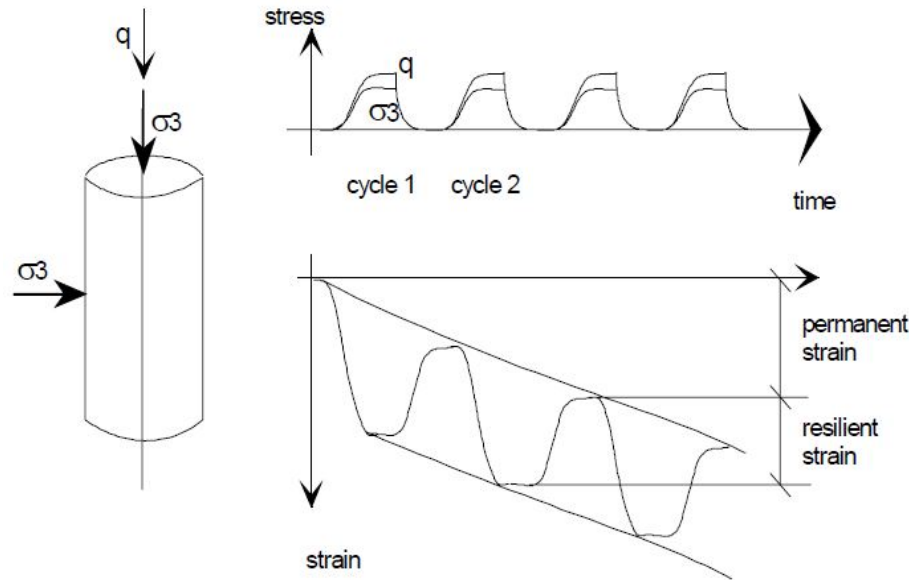


Repeated Load Triaxial Tests (RLTT)

to reproduce the traffic repeated solicitations



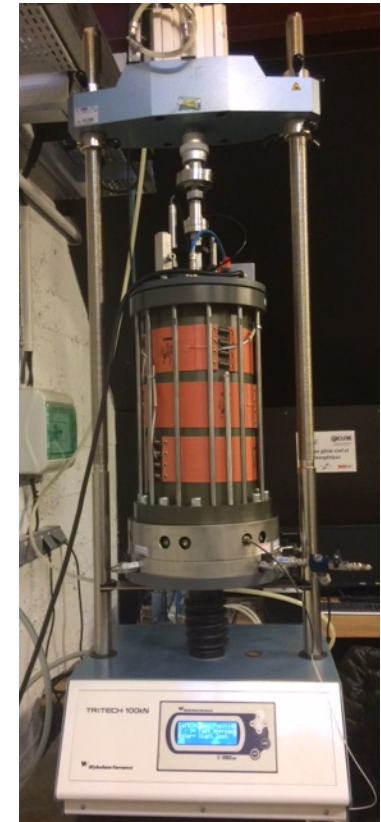
Triaxial cell at IFSTTAR



- Constant or variable confining pressure σ_3
- Cyclic deviator stress q

2 phases :

- **Conditioning:** stabilisation of permanent strains (30 000 cycles)
- **Resilient test:** study of resilient strains (100 cycles per stress path)



Temperature controlled triaxial cell at INSA

Why RLTT ?

PHASE 1 : conditioning

Permanent strains

Rutting resistance

PHASE 2 : resilient test

Resilient strains

Resilient modulus
 Characteristic Elastic modulus

French Standard for UGM NF EN 13286-7

Traffic category T3 : low traffic

Permission to use the material in base and subbase courses if :

Mechanical properties	Characteristic permanent axial strain ϵ_1^c	Characteristic elastic modulus E_c
OR	$2.5 \times 10^{-3} < \epsilon_1^c \leq 6 \times 10^{-3}$	$500 \text{ MPa} \leq E_c$
	$\epsilon_1^c \leq 6 \times 10^{-3}$	$250 \text{ MPa} \leq E_c < 500 \text{ MPa}$

E_c : characteristic elastic modulus is determined on stress path : $p_{\max} = 250 \text{ kPa}$ and $q_{\max} = 500 \text{ kPa}$

ϵ_1^c : characteristic permanent axial strain is measured at the end of conditioning (after 20 000 cycles)

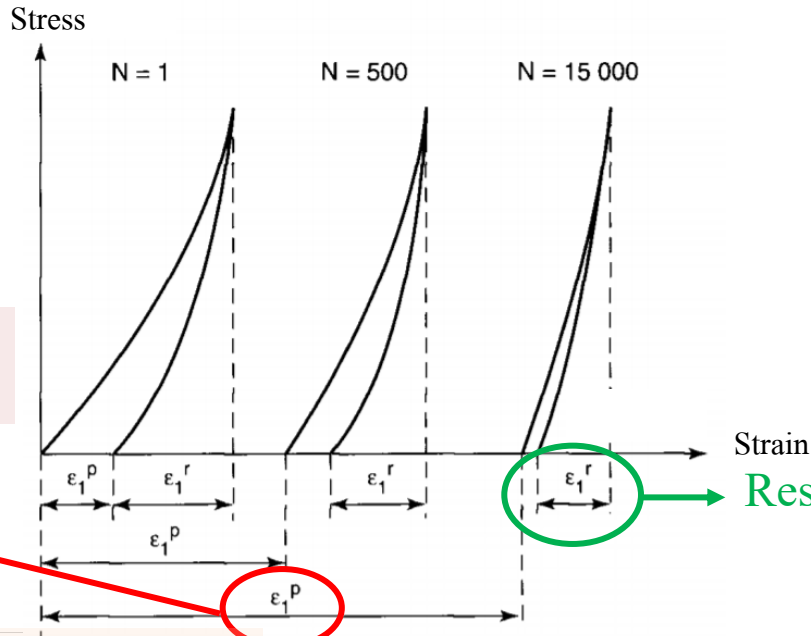
$$\epsilon_1^c = \epsilon_1^p(20\ 000) - \epsilon_1^p(100)$$

How ?

ϵ^p are stabilised if $\Delta\epsilon/\Delta N < 10^{-7}$
 (NF EN 13286-7, 2004)

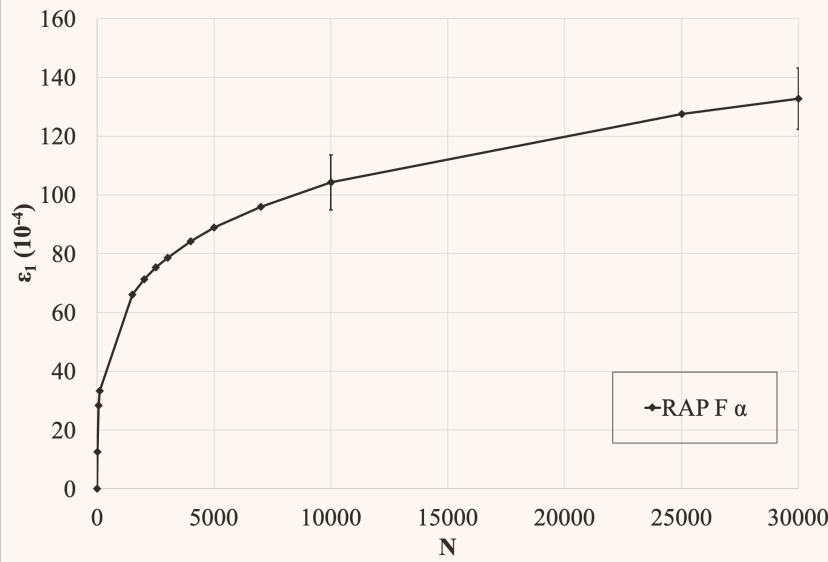
Permanent strain

Resilient strain

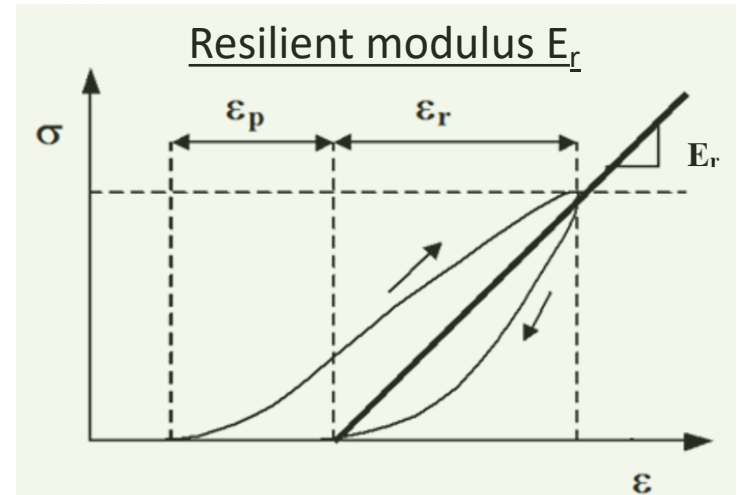


Permanent axial strains

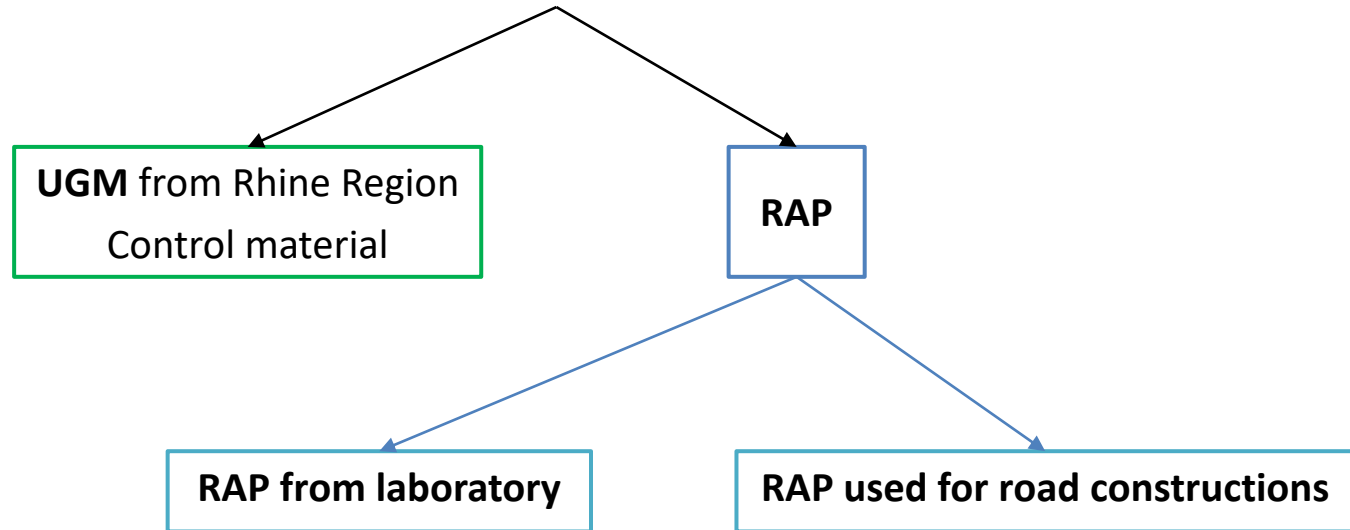
Balay et al. (1998)



Resilient modulus E_r



STUDIED MATERIALS



RAP F α : French RAP stock α (with PAH)

RAP F α -BE : after binder extraction

RAP F β : French RAP stock β (without PAH)

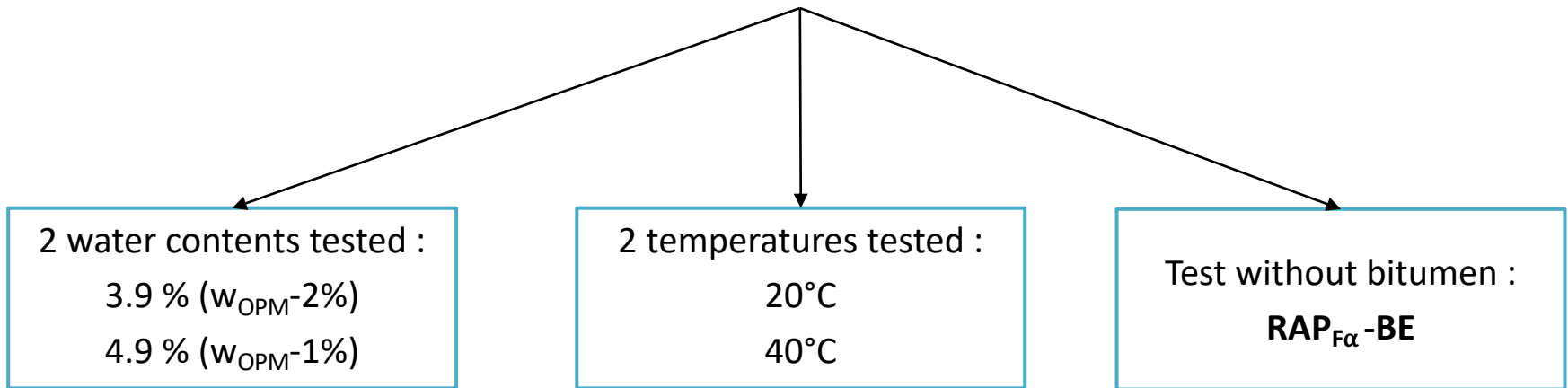
RAP CH : Swiss RAP

RAP F road construction : French RAP

RAP CH road construction : Swiss RAP

FOCUS ON

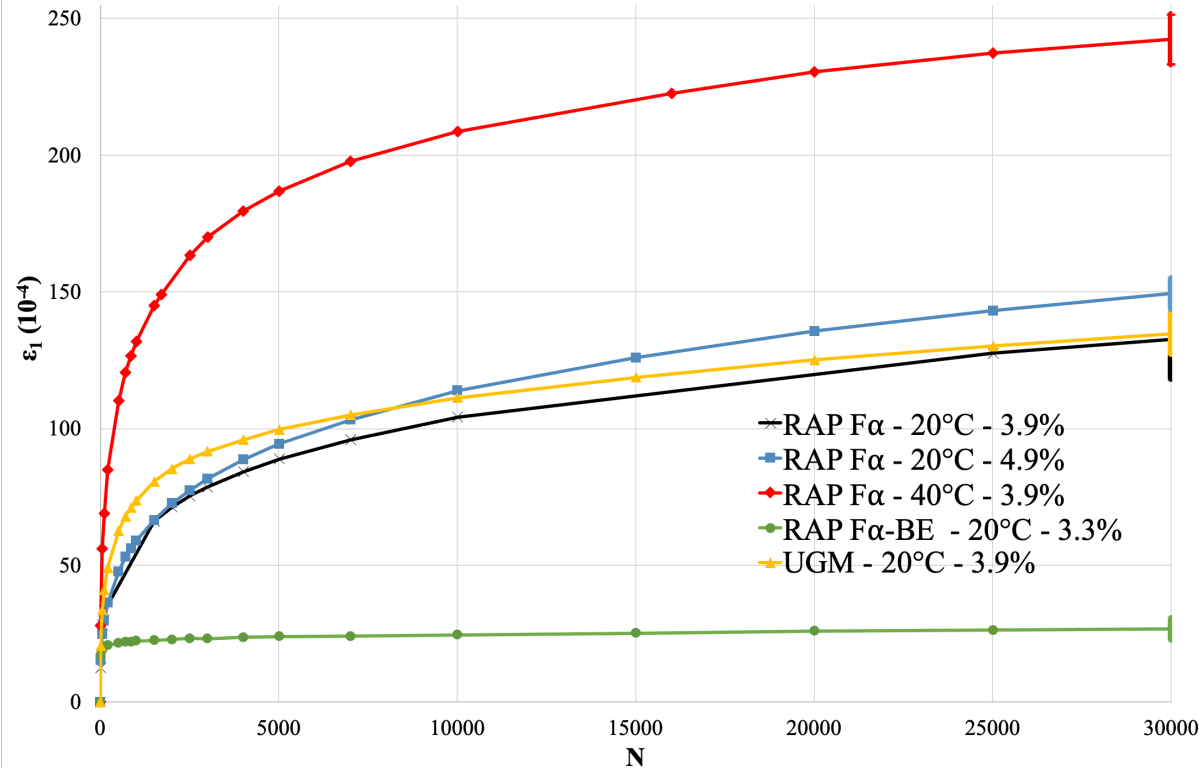
RAP F α : French RAP stock α (with PAH)



	Modified Proctor Test	
	w _{OMP} (%)	V _{d,OMP} (Mg/m ³)
RAP F α	5.9	2.02
RAP F α -BE	5.3	2.21
UGM	7.6	2.16

Rutting resistance

Permanent axial strains



Frequency : 2 HZ

T (°C)	Δq (kPa)	Δp (kPa)
20	340	113
40	200	67

Strain rate over the last hundred cycles

	$\Delta \epsilon_1 / \Delta N$
RAP Fα - 20°C – 3.9%	$1,04 \cdot 10^{-7}$
RAP Fα - 20°C – 4.9%	$1,26 \cdot 10^{-7}$
RAP Fα - 40°C – 3.9%	$1,01 \cdot 10^{-7}$
RAP Fα - BE - 20°C – 3.3%	$8,85 \cdot 10^{-9}$
UGM- 20°C – 3.9%	$9,06 \cdot 10^{-8}$

	Temperature (°C)	Water content (%)	Dry density γ_d (Mg/m ³)	Void content	Characteristic permanent axial strain ϵ_1^c (10 ⁻⁴)
RAP F α	20	3.9	1.96	0.27	133
RAP F α	20	4.9	1.96	0.27	150
RAP F α	40	3.9	1.96	0.27	242
RAP F α -BE	20	3.3	2.14	0.24	27
UGM	20	3.9	1.96	0.35	135

- Permanent strains increase with water content
- Permanent strains increase with temperature \longrightarrow due to the viscosity of bitumen
- RAP and UGM : similar levels of strain
- RAP-BE (after binder extraction) : very low strains \longrightarrow due to the high fines content of RAP-BE

Elastic modulus

q_{max} (kPa)	p_{min} (kPa)
500	250

	Temperature (°C)	Water content (%)	Dry density γ_d (Mg/m ³)	Void content	Characteristic elastic modulus at 0.1 Hz E_c (MPa)
RAP F α	20	3.9	1.96	0.27	475
RAP F α	20	4.9	1.96	0.27	342
RAP F α	40	3.9	1.96	0.27	349
RAP F α -BE	20	3.3	2.14	0.24	445
UGM	20	3.9	1.96	0.35	222

The RAP F α presents a higher level of elastic modulus than UGM

FOCUS ON different sources of RAP

RAP from laboratory

RAP used for road constructions

RAP F α : French RAP stock α (with PAH)

RAP F β : French RAP stock β (without PAH)

RAP CH : Swiss RAP

RAP F road construction : French RAP

RAP CH road construction : Swiss RAP

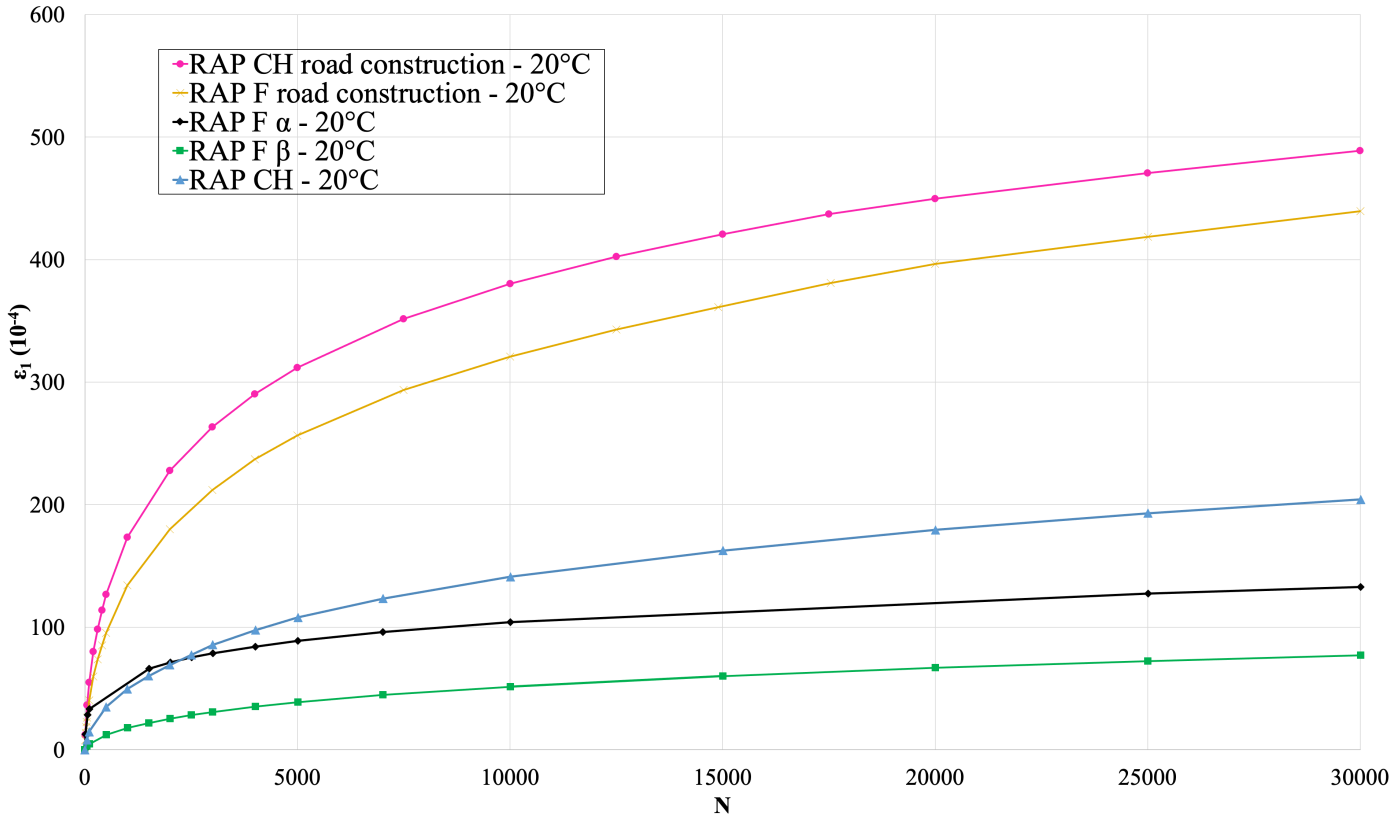
	Modified Proctor Test	
	w _{OMP} (%)	$\gamma_{d,OMP}$ (Mg/m ³)
RAP F α	5.9	2.02
RAP F β	5.5	1.79
RAP CH	4.3	1.81
RAP F road construction	7.2	1.96

Rutting resistance

Permanent axial strains

Frequency : 2 HZ

Δq (kPa)	Δp (kPa)
340	113



Strain rate over the last hundred cycles $\Delta\varepsilon_1/\Delta N$

RAP Fα – 3.9%	$1,04 \cdot 10^{-7}$
RAP Fβ – 3.5%	$3,57 \cdot 10^{-8}$
RAP CH – 3.7%	$1,81 \cdot 10^{-7}$
RAP F road construction – 3.9%	$3,17 \cdot 10^{-7}$
RAP CH road construction – 3.7%	$3,83 \cdot 10^{-7}$

Elastic modulus

q_{\max} (kPa)	p_{\min} (kPa)
500	250

	Temperature (°C)	Water content (%)	Dry density γ_d (Mg/m ³)	Characteristic elastic modulus at 0.1 Hz E_c (MPa)
RAP F α	20	3.9	1.96	475
RAP F β	20	3.5	1.95	551
RAP CH	20	3.7	1.96	505
RAP F road construction	20	3.9	1.90	425
RAP CH road construction	20	3.7	1.87	372

The elastic moduli of RAP correspond to a good quality UGM

Conclusions

➤ According to the French Standard for UGM NF EN 13286-7 :

Traffic category T3 : low traffic

Permission to use the material in base and subbase courses if :

Mechanical properties	Characteristic permanent axial strain ϵ_1^c	Characteristic elastic modulus E_c
OR	$2.5 \times 10^{-3} < \epsilon_1^c \leq 6 \times 10^{-3}$	$500 \text{ MPa} \leq E_c$
	$\epsilon_1^c \leq 6 \times 10^{-3}$	$250 \text{ MPa} \leq E_c < 500 \text{ MPa}$



The permanent strain criterion is not checked

Solutions :

Positive evolution in the long term
Increase the compaction density

The modulus criterion is checked



These results confirm that the RAP can be used instead of UGM, in base and subbase layers in low-traffic roads: an alternative to deposit solution for contaminated RAP

ACKNOWLEDGMENTS

- The ORRAP project is supported by the INTERREG Upper Rhine program from the ERDF (European Regional Development Fund) to the tune of 622 553€ and by the Swiss Confederation, Canton Basel-Landschaft Canton Aargau.
- ORRAP started in November 2016 with a total budget of 1.48 M€. Due to ongoing experimental requirements, its duration has been extended until December 2020.





Recyclage optimal des agrégats de béton bitumineux dans les chaussées à faible trafic
Optimales Recycling von Ausbauasphalt auf verkehrsschwachen Straßen

01/11/2016 - 31/12/2020

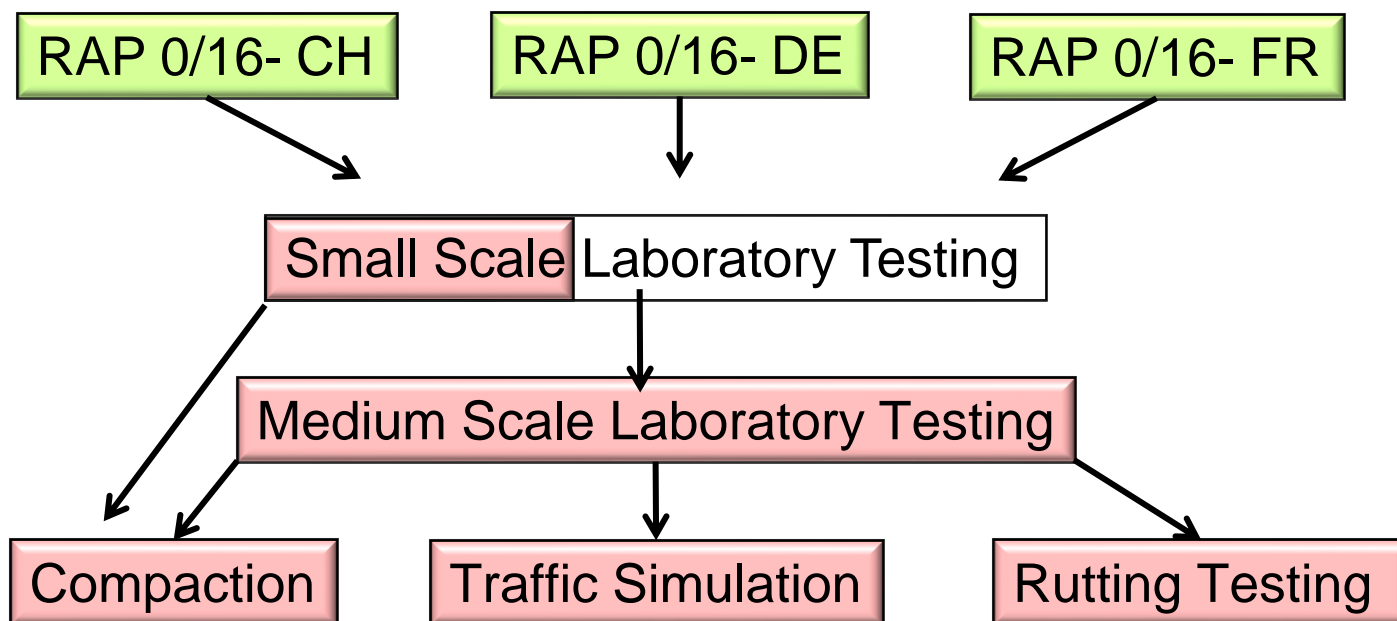
FINAL ANNUAL MEETING

02/12/2020

Medium size specimens compaction and testing

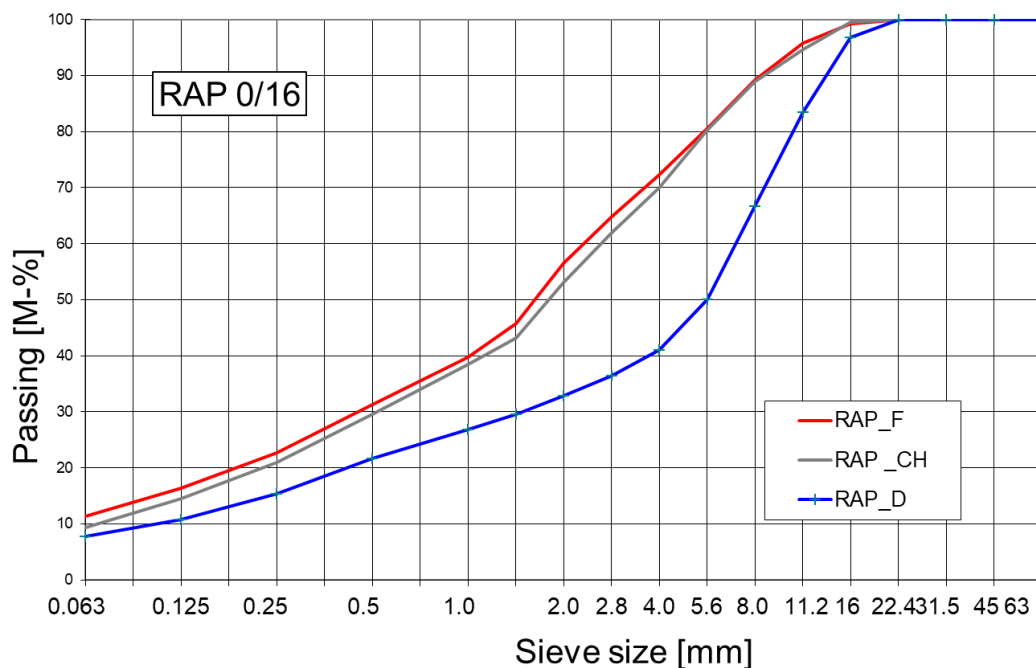
Christiane Raab, Empa

Empa's contribution



RAP Material

RAP Type	RAP Source	RAP Properties			RAP Binder Properties	
		Bit. Cont. [w%]	Density [kg/m ³]	Water Cont. [w%]	Pen [0.1mm]	R&B [°C]
RAP 0/16	CH	6.2	2.371	1.6	17	69.0
	DE	4.3	2.347	3.5	17	68.4
	FR	6.3	2.408	5.0	16	69.4



Compaction: Small RAP-CH Specimens

Parameters

- 2 Types: Marshall & Gyratory
- 2 Compaction Efforts
- 4 Temperatures

Type	Effort [Number]		RAP Temperature [°C]			
	50	100	20	60	80	100
Marshall [Blows]	50	100	20	60	80	100
Gyratory [Gyrat.]	204	410	20	60	80	100



0/16					
Temperature		Marshall Test		Gyrator Test	
		50 Blows	100 Blows	205 Gyrat.	410 Gyrat.
Temperature	Room (20°C)	Impossible	Too Weak	Good Enough	Good Enough
	80°C	Good	Good	Very Good	Very Good
	100°C	Good	Good	Very Good	Very Good

Compaction: Small RAP-CH Specimens

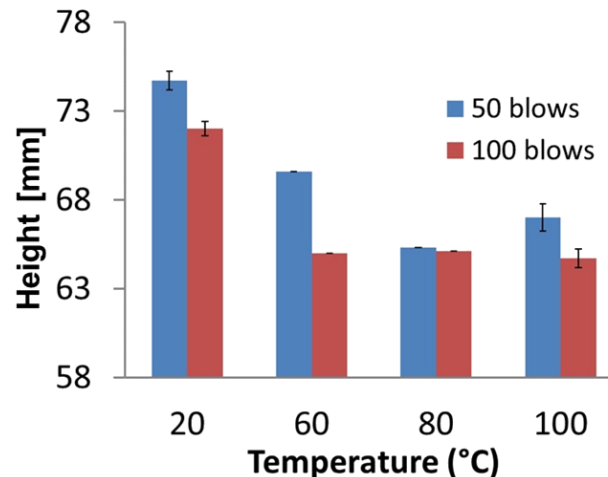
Parameters

- 2 Types: Marshall & Gyratory
- 2 Compaction Efforts
- 4 Temperatures

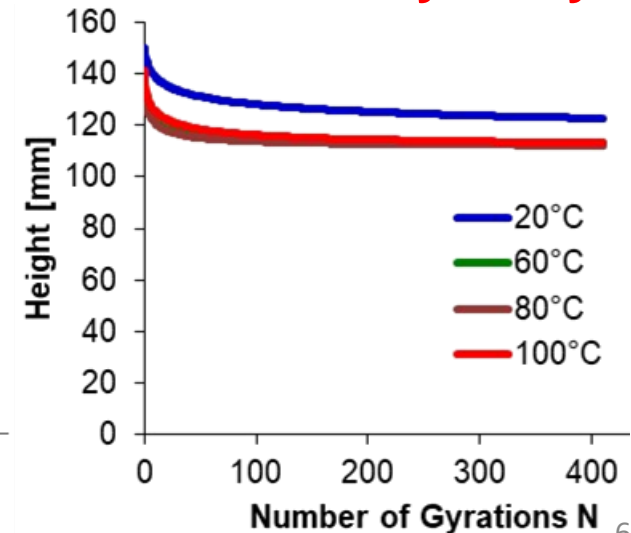
Type	Effort [Number]		RAP Temperature [°C]			
	50	100	20	60	80	100
Marshall [Blows]	50	100	20	60	80	100
Gyratory [Gyrat.]	204	410	20	60	80	100

60°C seems OK with more compaction effort

Marshall



Gyratory



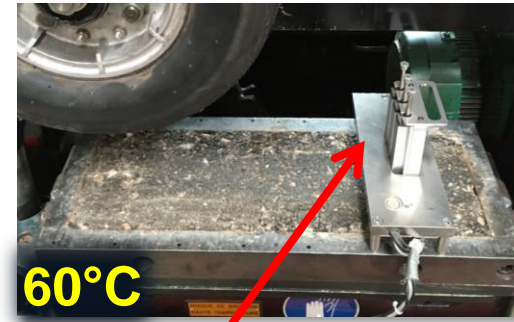
Compaction: Medium Size RAP Specimens

- Large Wheel Rut Test
- Specimen Size: 500 x 180 x 50 (100) mm

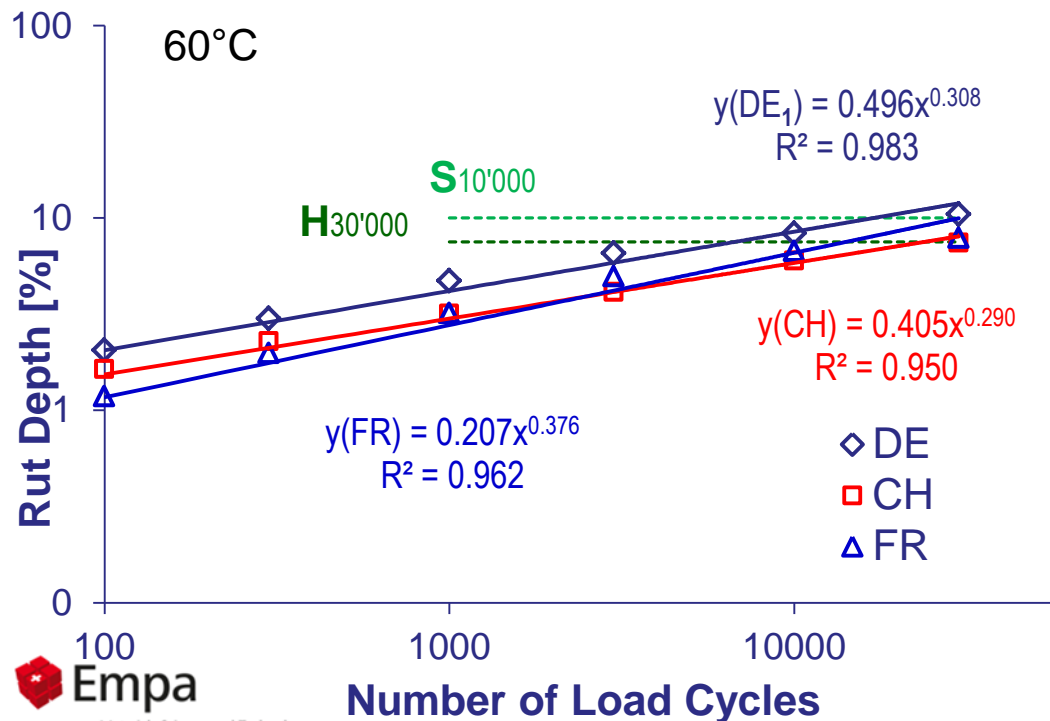


Large Wheel Rutting Test Results

- Mean Values (2 Tests, 5 Profiles)
- 30'000 Cycles, T=60°C



Profile Meas. LVDT Table



- CH → OK for Very Heavy Traffic <7.5% @ 30'000 Cycles (H)
 - FR,(DE₁) → OK for Heavy Traffic <10% @ 10'000 Cycles (S)
- (According to CH Standard)

Compaction: Medium Size RAP Specimens

- Plate Compactor
- Specimen Size: 1200 x 400 x 50 mm



Compaction: Medium Size RAP Specimens

- Static Roller Compactor
- Specimen Size: 1300 x 430 x 65 mm

During Compaction

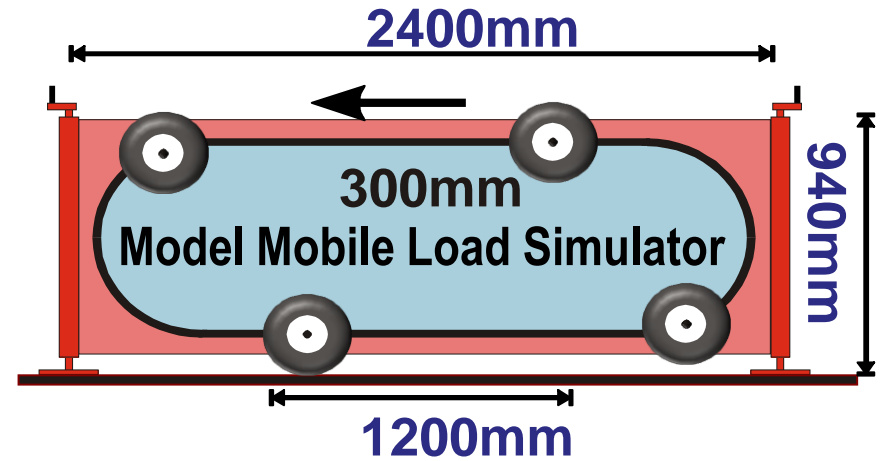


Aggregate Loss: ca. 0.1% by weight

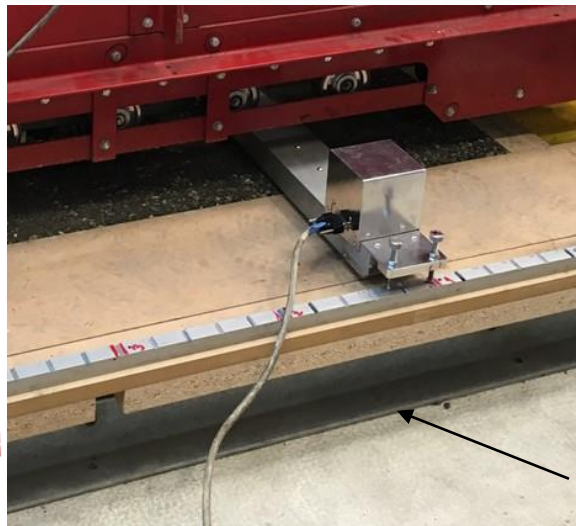
Specimen after compaction: surface



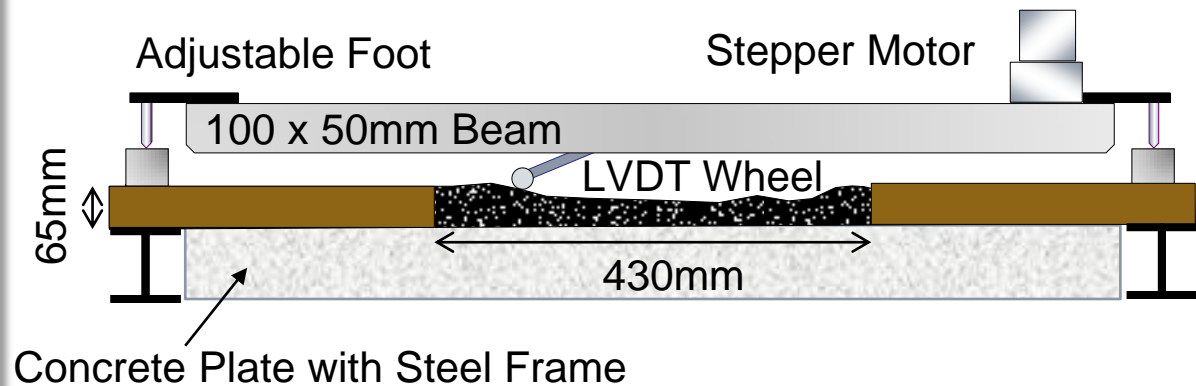
MMLS3: Lab-Scaled Traffic Simulator Testing



N=7200 Passg/h; v=9.4km/h= 2.6 m/s
50'000 Passings; F=2.1kN; p=600kPa

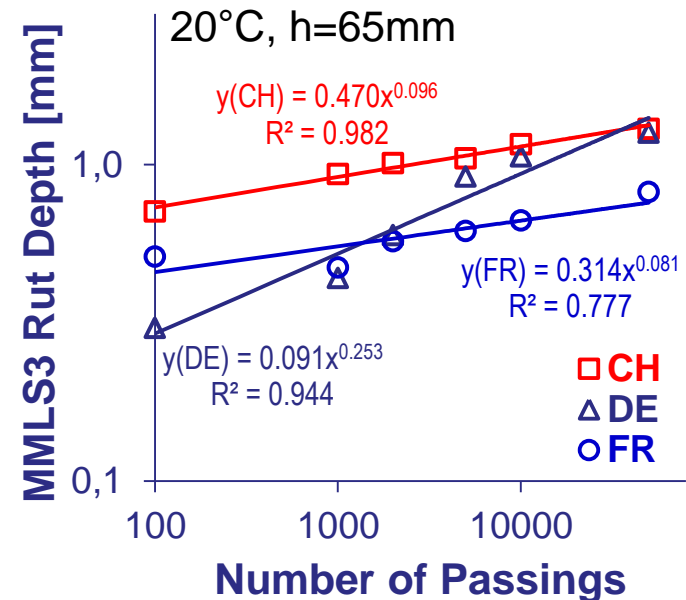
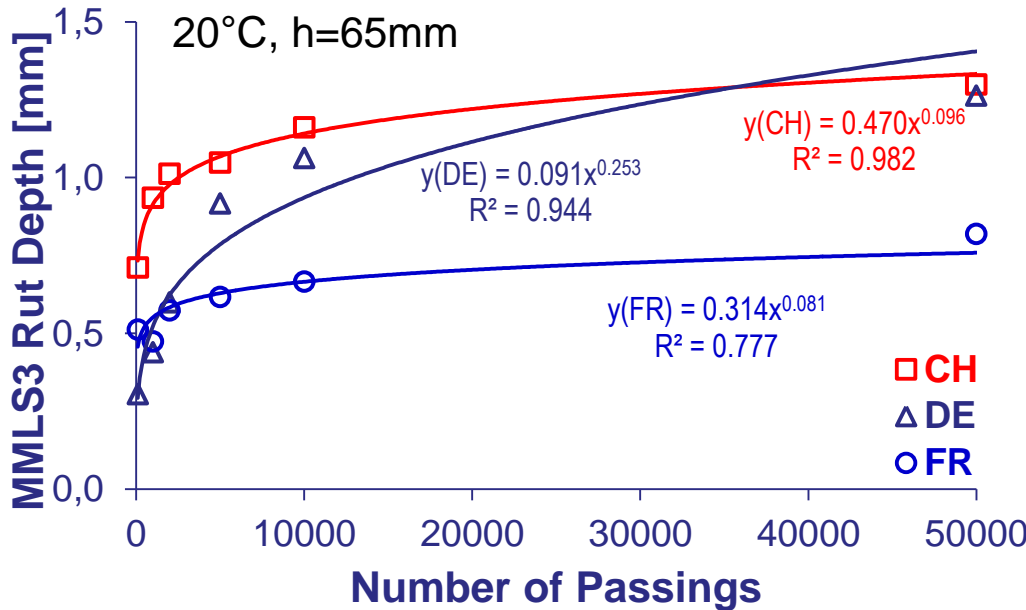
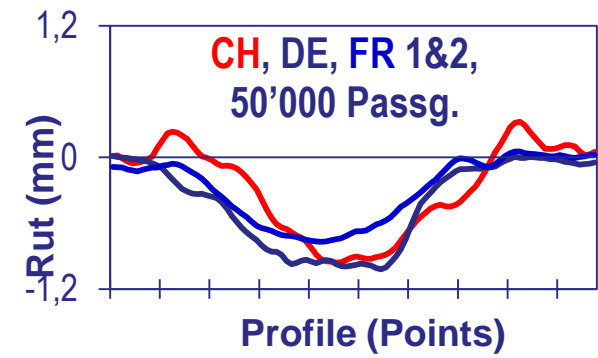
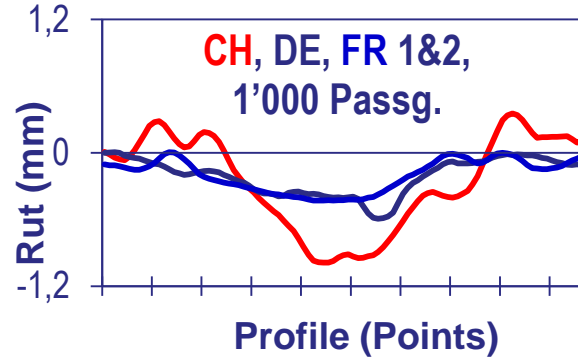


Profilometer



MMLS3 Test Results

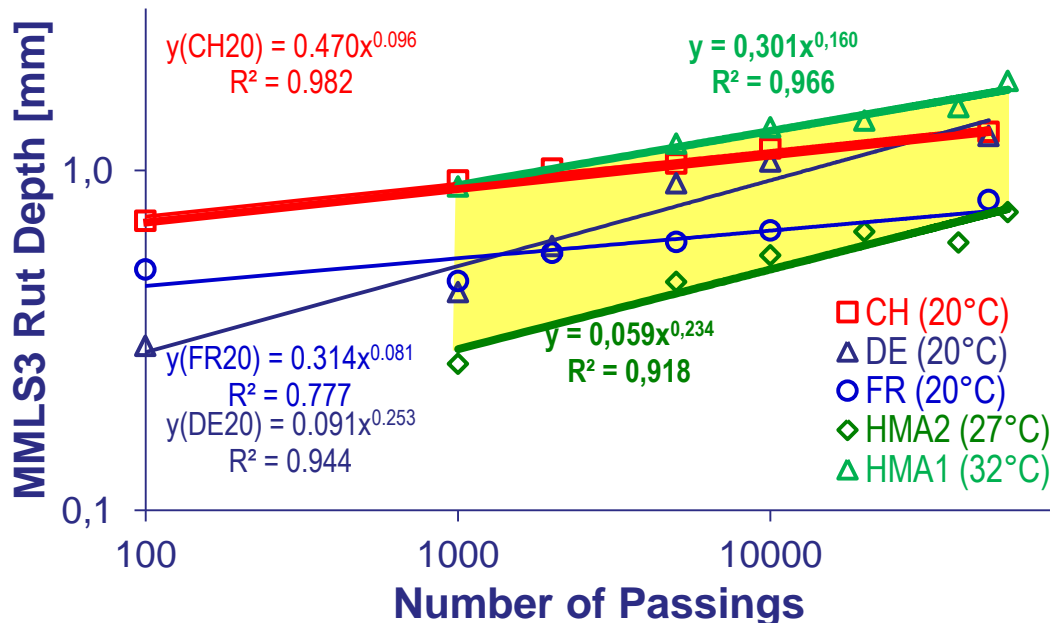
- Mean Values (2 Tests, 3 Profiles)
- 50'000 Cycles, **T=20°C**



- CH → Most Rutting at Beginning, then **Stable**
- DE → **Fastest** Rutting Speed, **Not Stable**
- FE → **Least** Rutting, Long-term **Stable** like CH

Rutting Results Discussion

- Comparison with other MMLS3 Tests in Situ on **Hot Mix Asphalt AC8L**:
 - **HMA1** no RAP (Test @ 32°C)
 - **HMA2** with 80%RAP (Test @27°C)



- Rutting Similar to **HMA AC8L**
- Generally: **Little** Rutting

- Comparing Large Wheel Rutting Test (LWRT) & MMLS3:
 - Generally: **FR** was Good, **DE** was not so Good
 - **FR** best @ 20°C (MMLS3), **CH** best @ 60°C (LWRT)

Conclusions

- **Lab compaction** of small and medium size specimens at **20°C** not sufficient, **but** increasing the temperature very little (**60°C**) changed compaction behavior drastically and made it possible to **construct stable and rutting resistant asphalt aggregate samples**.
- Rutting resistance with **large wheel rutting tester at 60°C** and **scaled traffic simulator MMLS3** at ambient temperature, both under laterally confinement of the specimens, produced **low rut depth**. For RAP-CH rutting resistance (30'000 cycles) with **large wheel device** even fulfilled the **requirements for very heavy trafficked roads**. All RAP fulfill requirements for (10'000 cycles) **heavy traffic**.
- The investigation clearly showed the **potential of using asphalt aggregates at low compaction temperatures** and was therefore very promising for a **successful in situ installation** of these materials.

ACKNOWLEDGMENTS

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Thank you for you attention